

# CE 501: Surface Water Hydrology



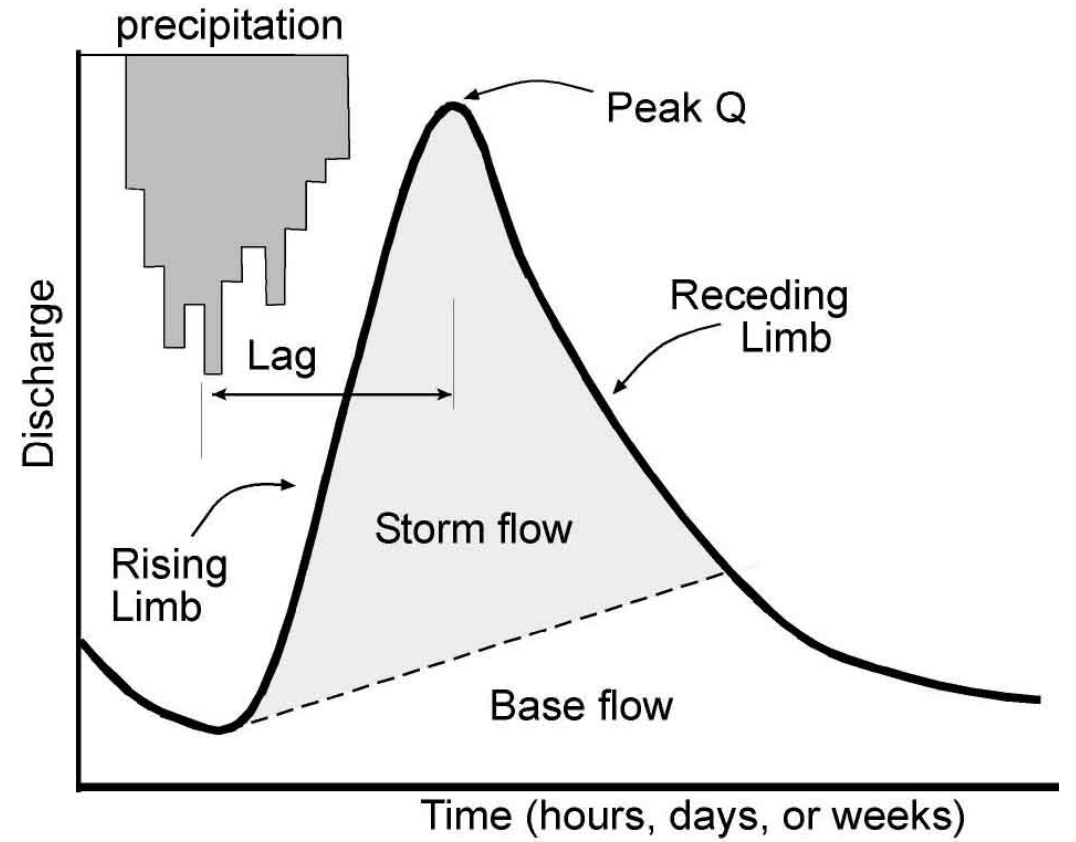
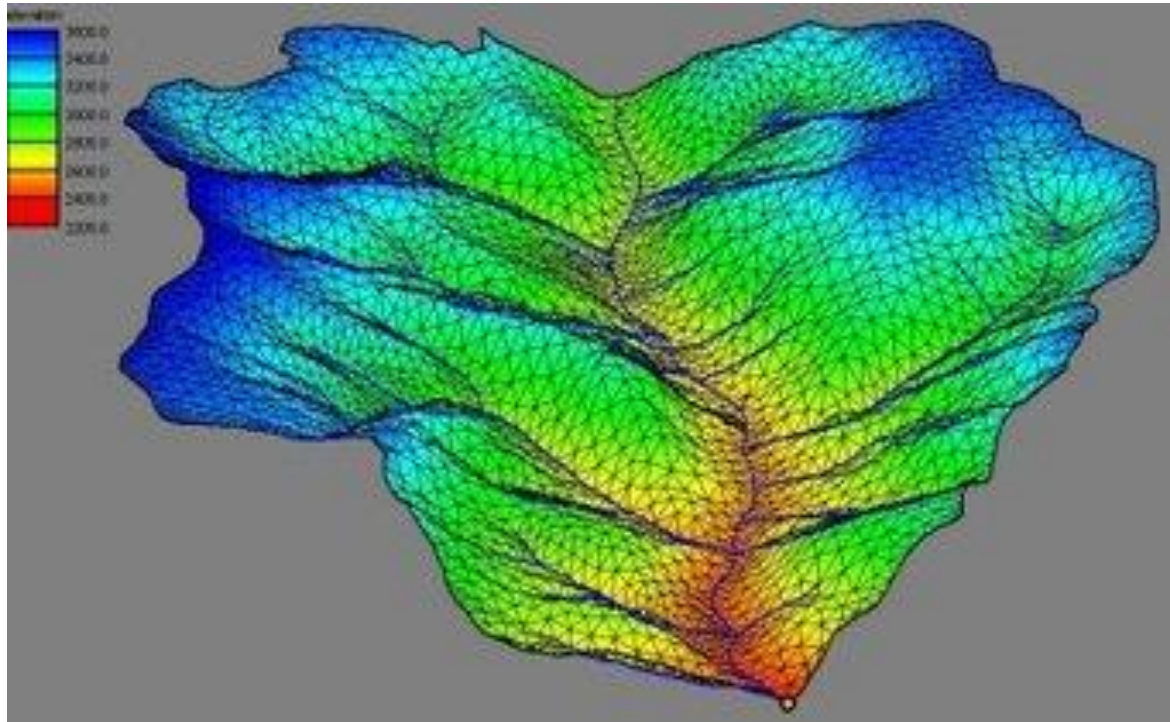
**Prof. (Dr.) Rajib Kumar Bhattacharjya**

Indian Institute of Technology Guwahati

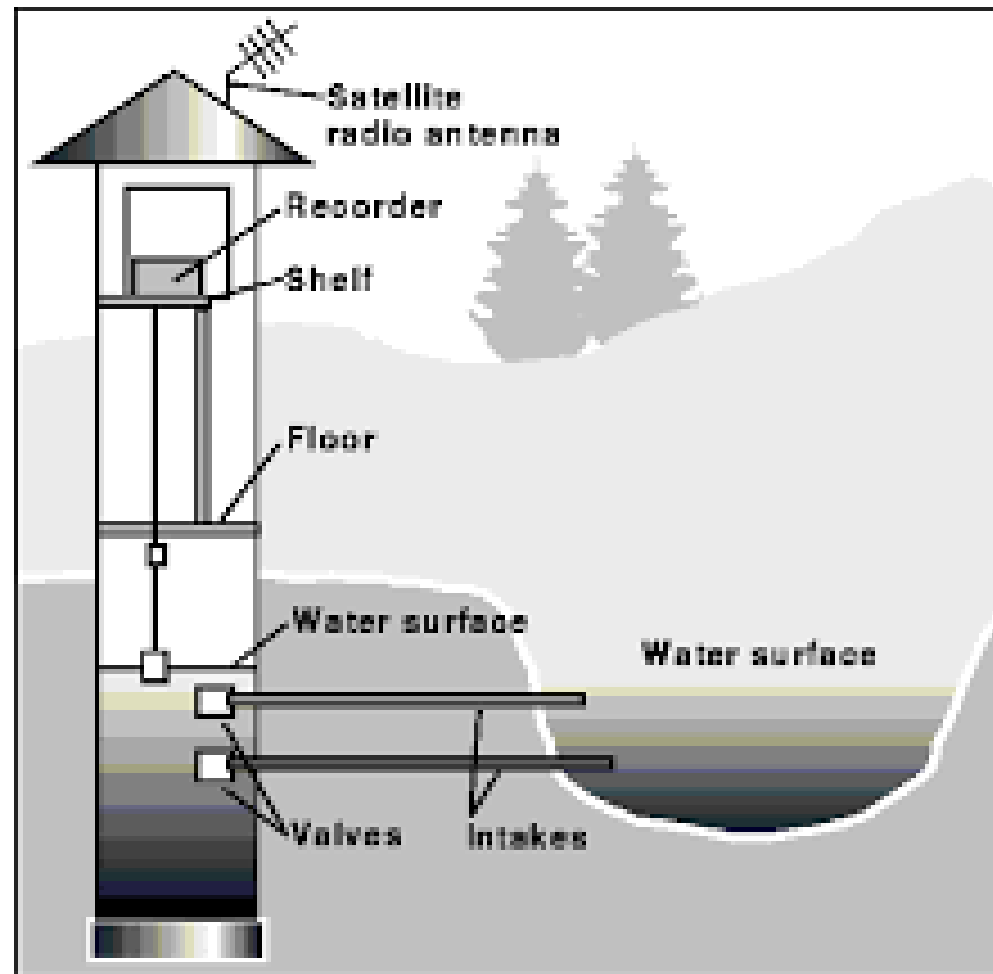
Guwahati, Assam

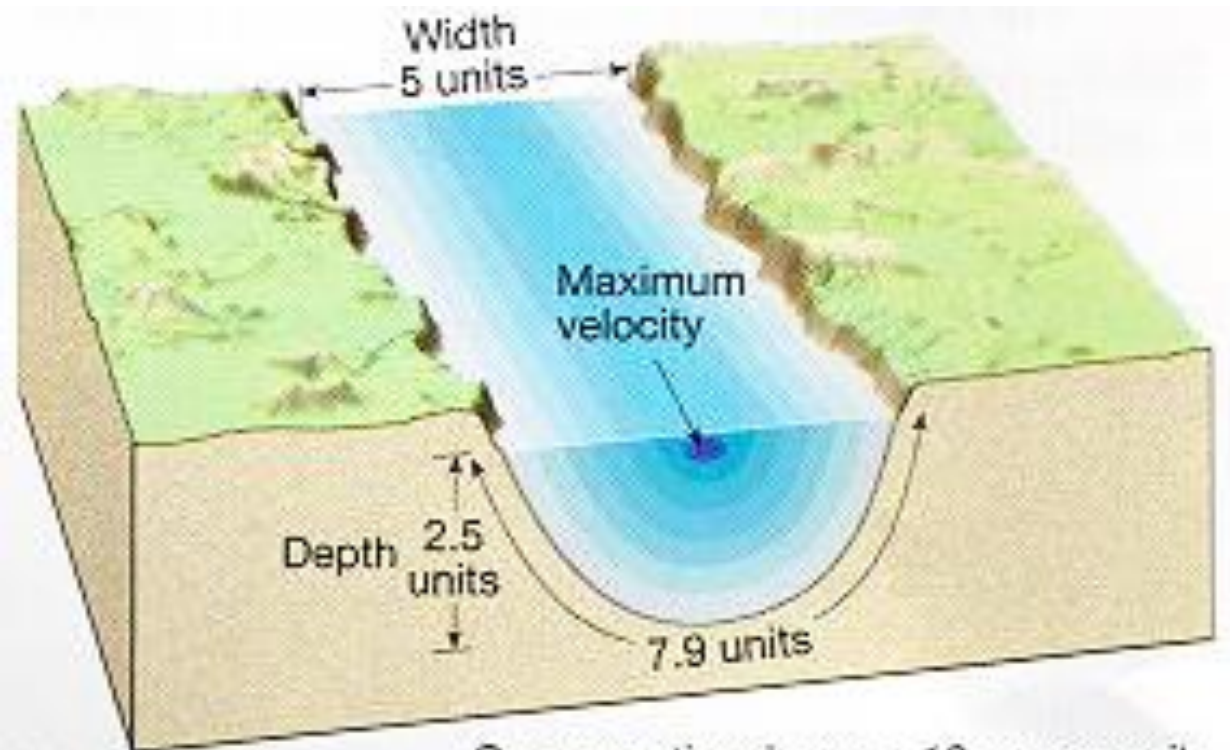
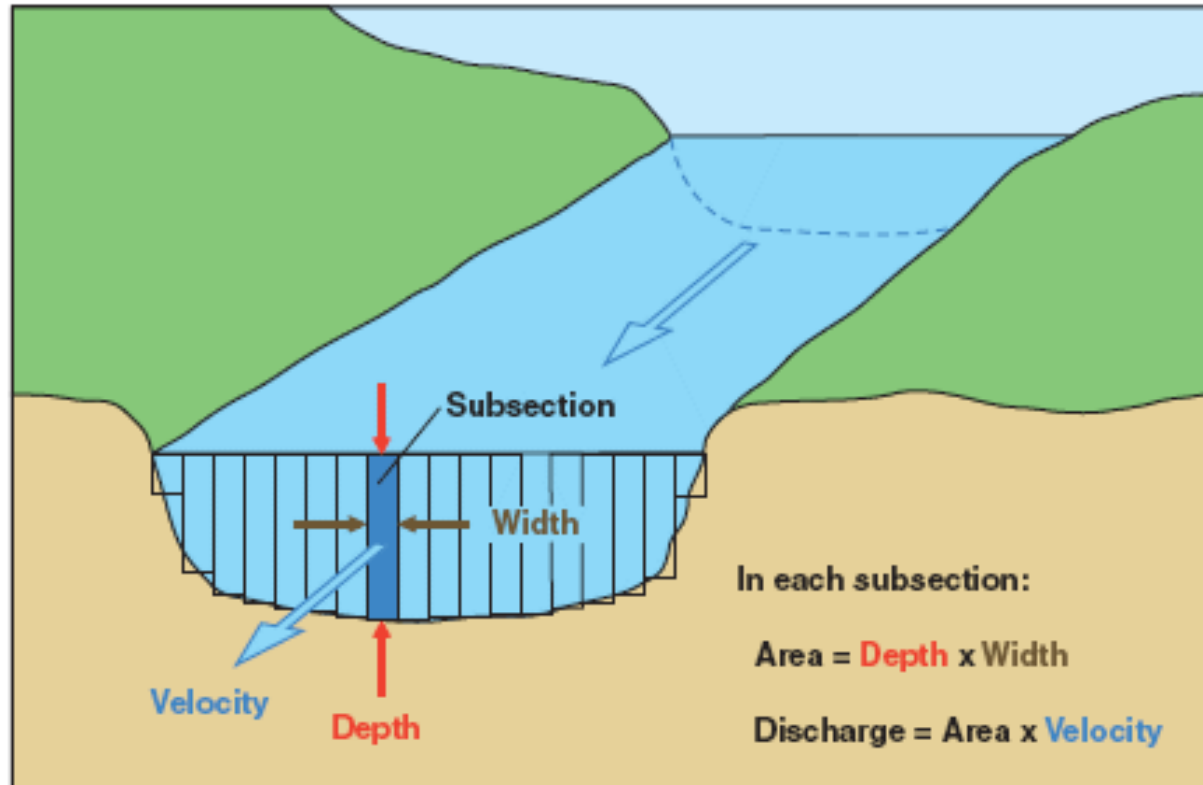
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# Stream gauging station



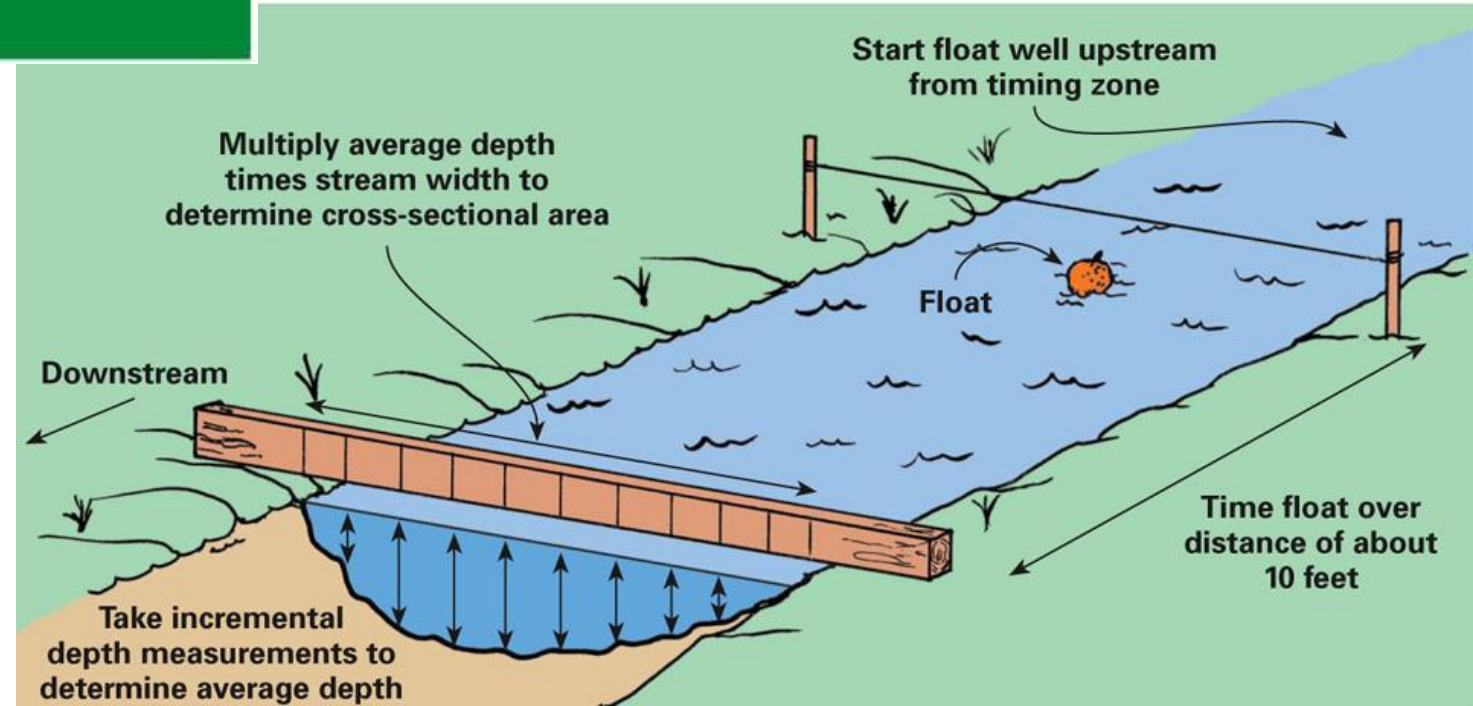
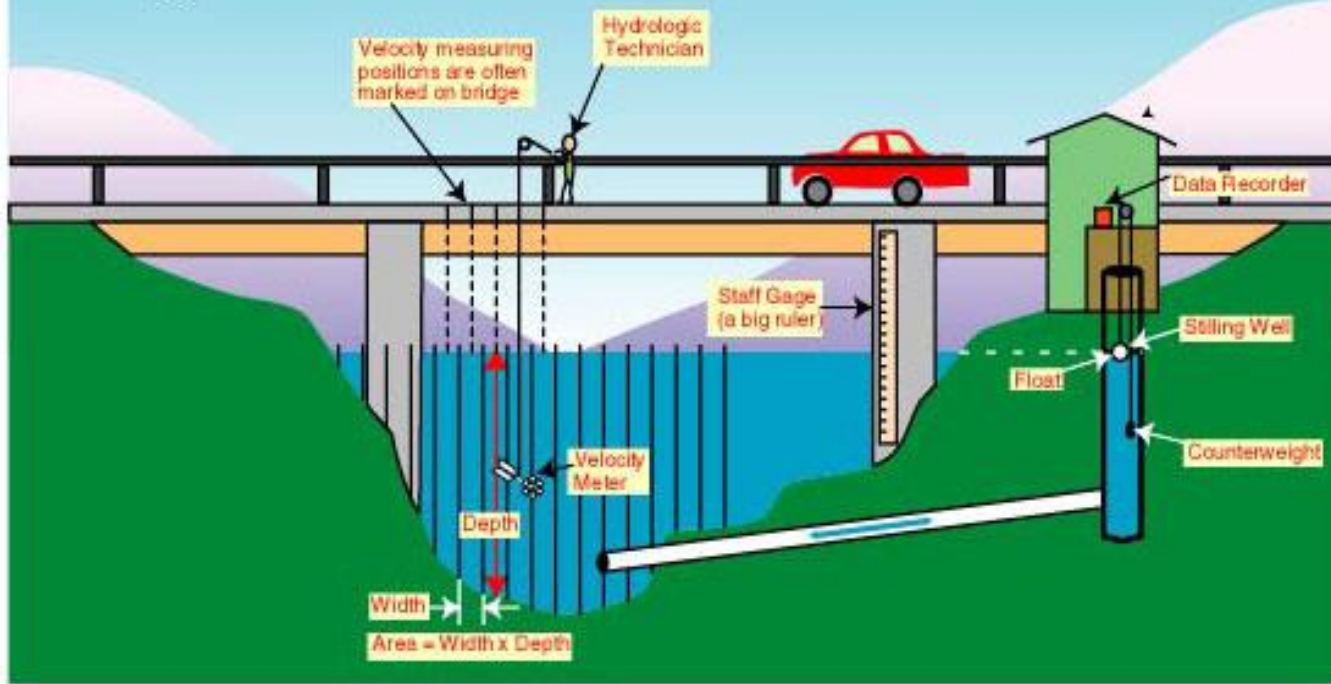


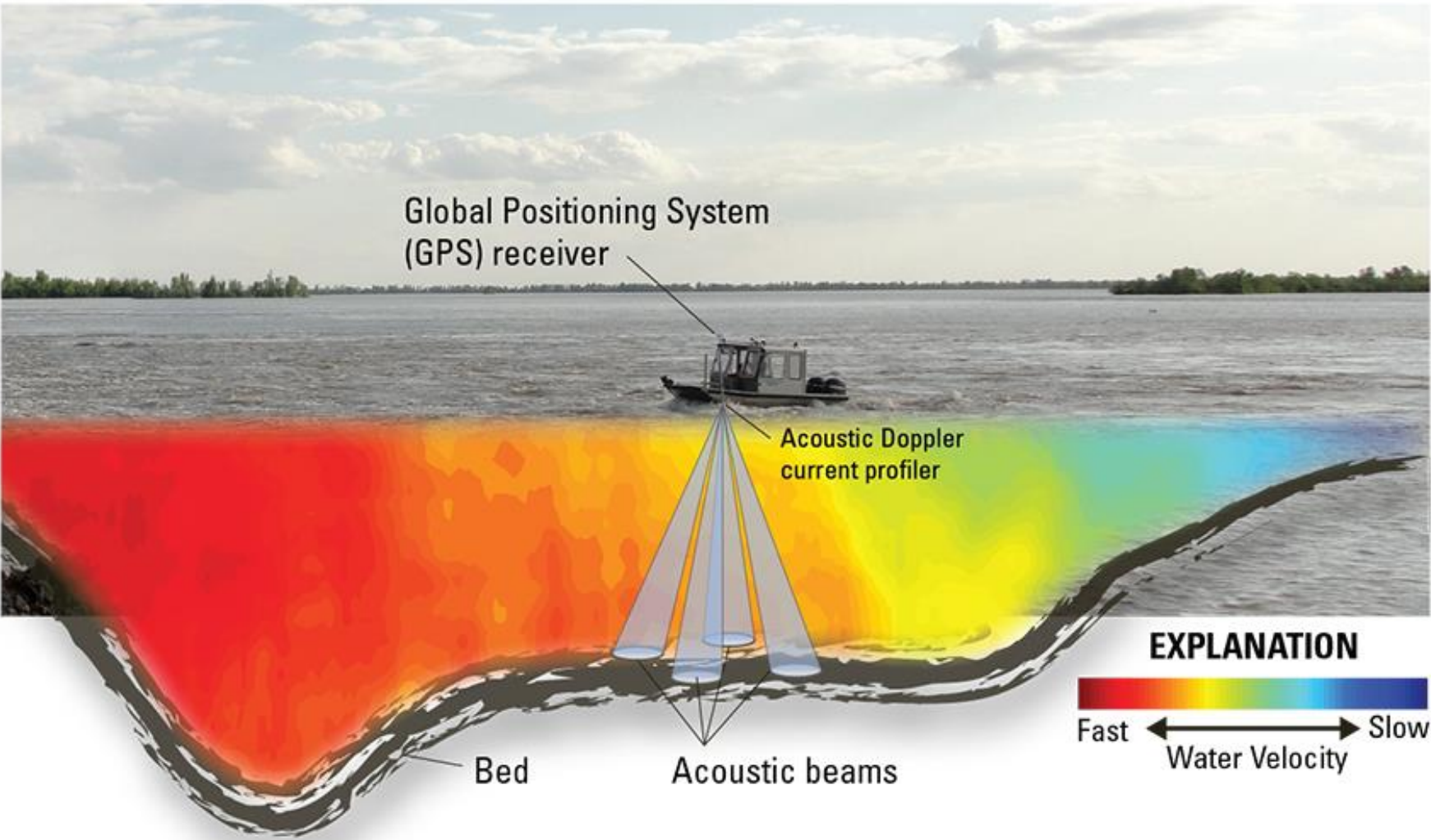
B. Semicircular channel

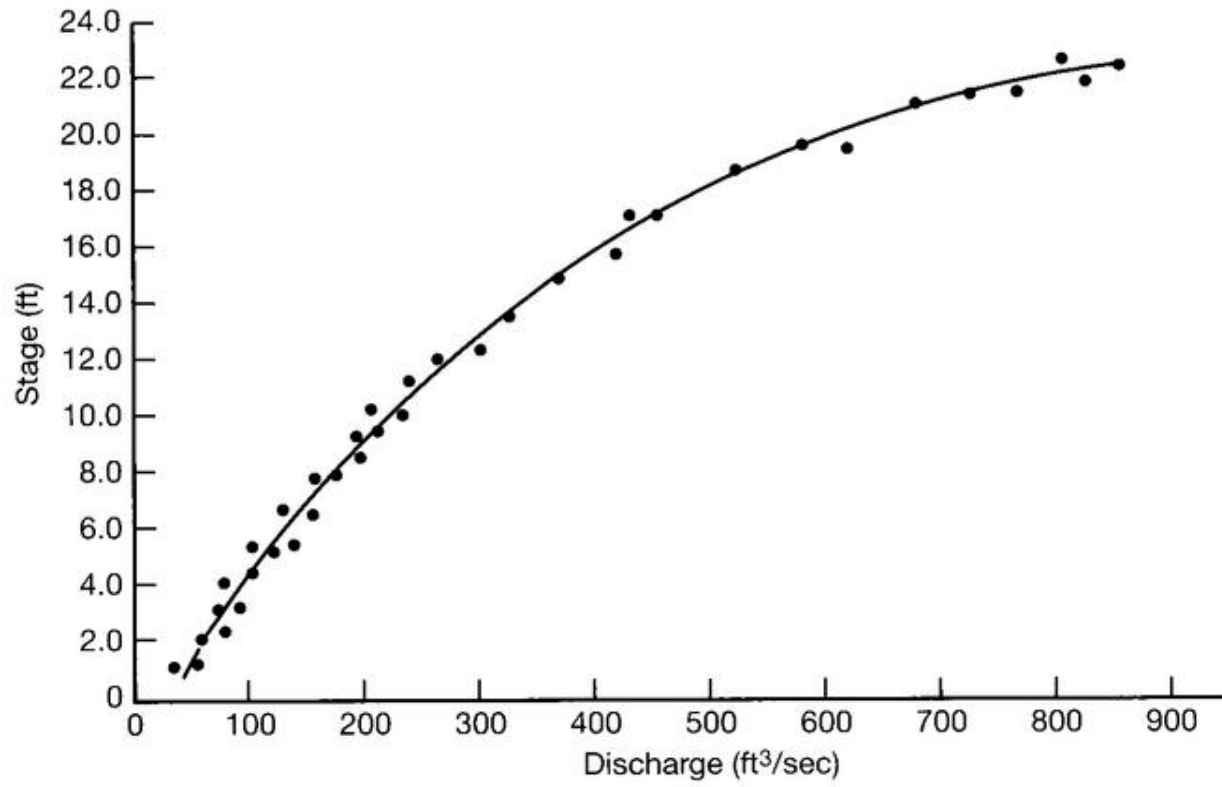
Cross-sectional area = 10 square units  
 Perimeter = 7.9 units

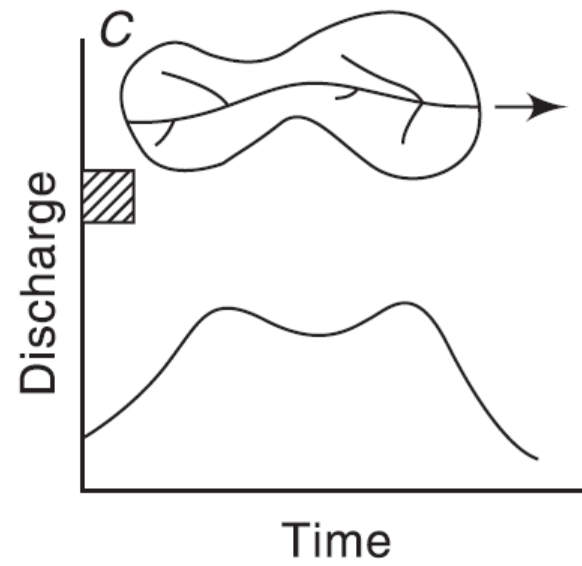
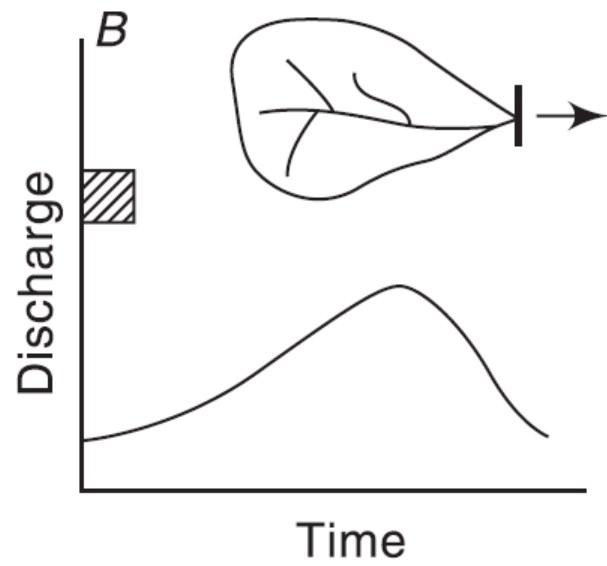
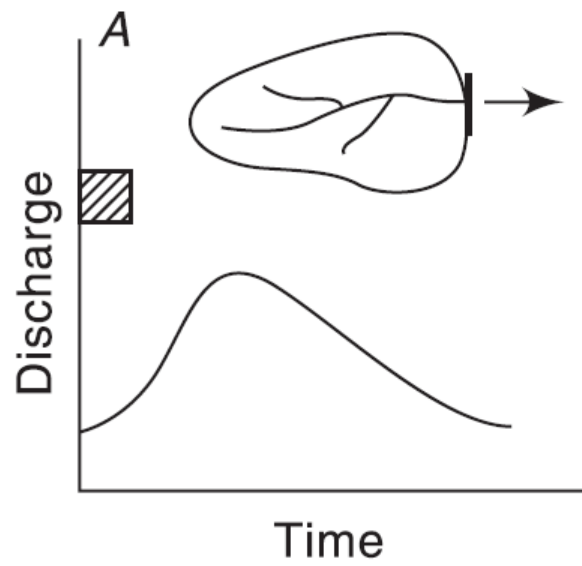


# TYPICAL RIVER CROSS SECTION

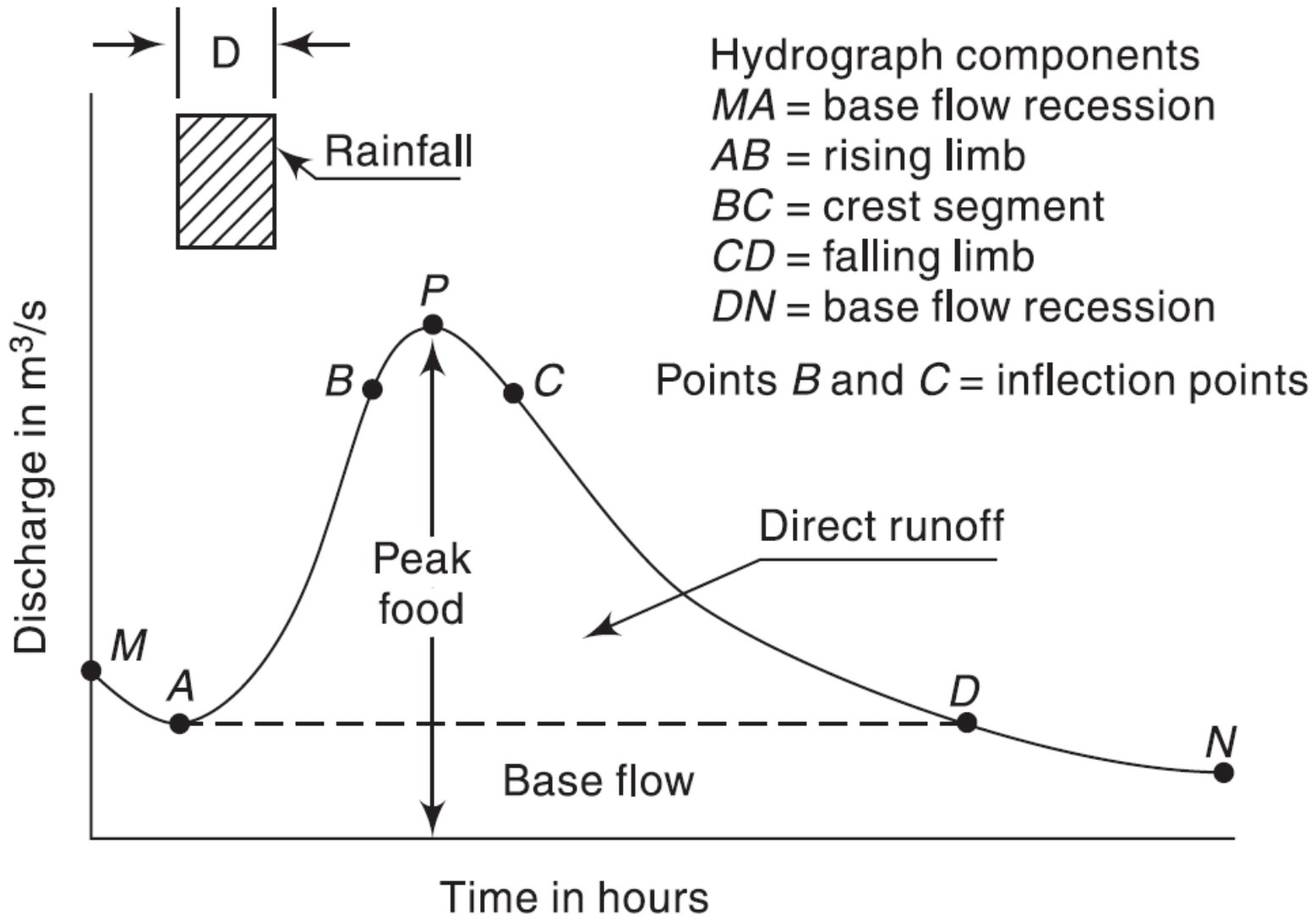








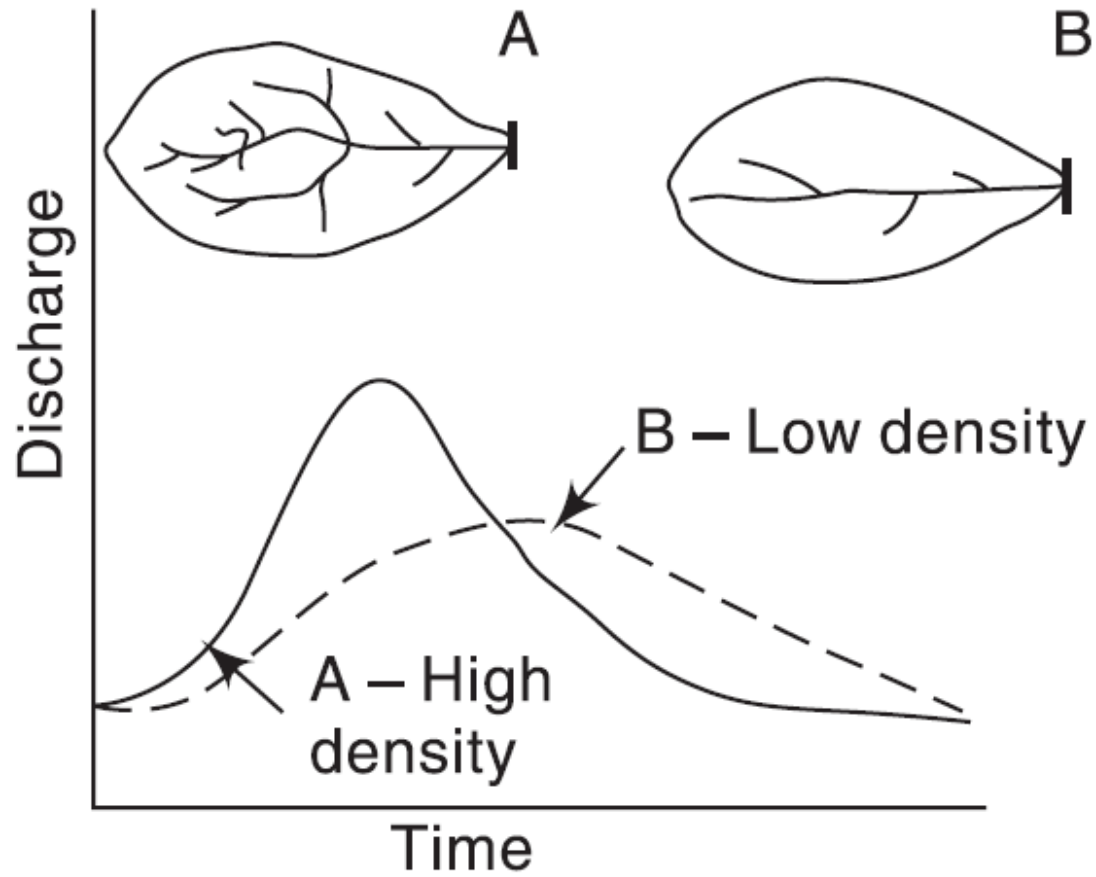




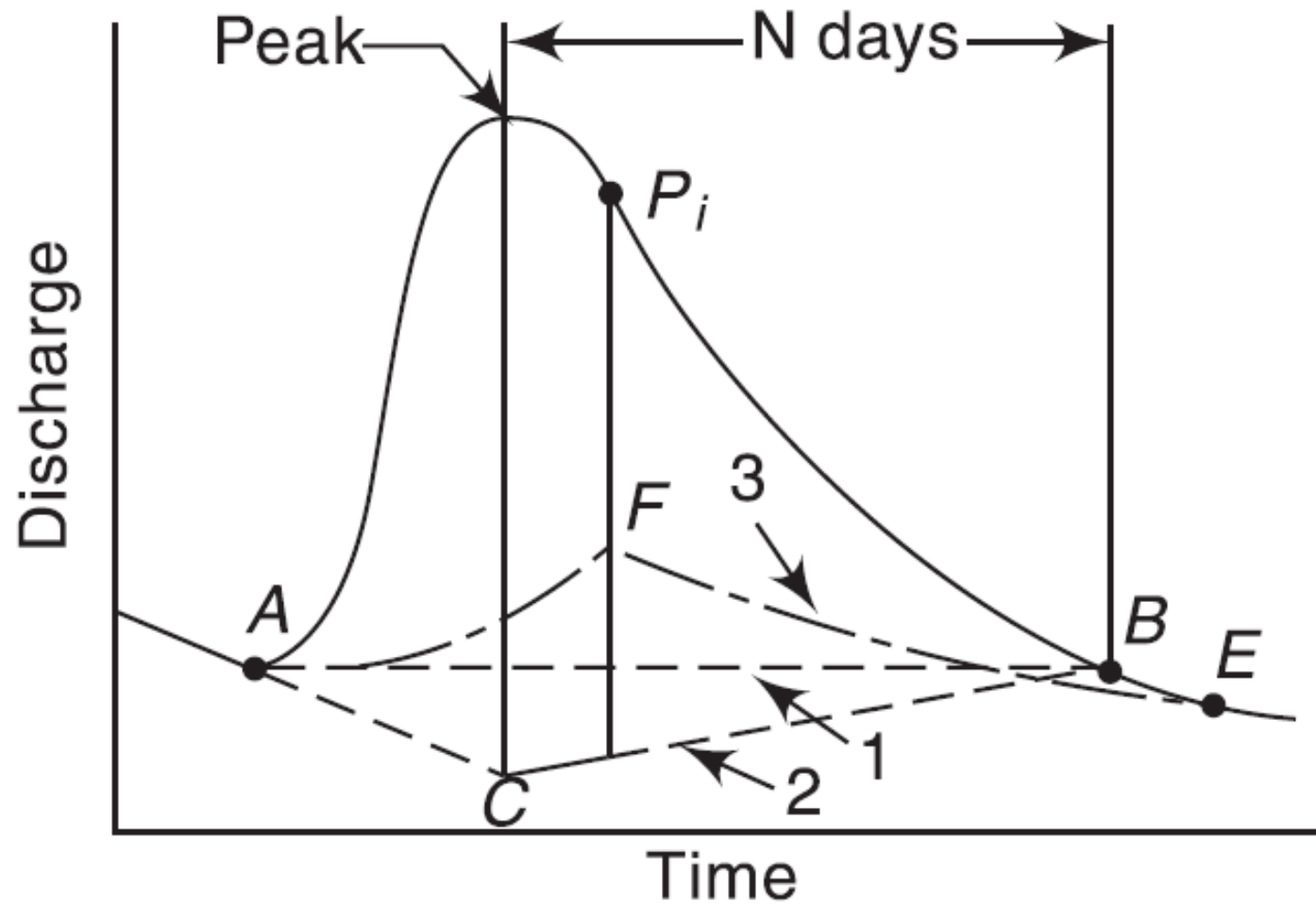
**Fig. 6.1** Elements of a Flood Hydrograph

**Table 6.1** Factors Affecting Flood Hydrograph

<b>Physiographic factors</b>	<b>Climatic factors</b>
<ol style="list-style-type: none"><li>1. Basin characteristics:<ol style="list-style-type: none"><li>(a) Shape</li><li>(b) size</li><li>(c) slope</li><li>(d) nature of the valley</li><li>(e) elevation</li><li>(f) drainage density</li></ol></li><li>2. Infiltration characteristics:<ol style="list-style-type: none"><li>(a) land use and cover</li><li>(b) soil type and geological conditions</li><li>(c) lakes, swamps and other storage</li></ol></li><li>3. Channel characteristics: cross-section, roughness and storage capacity</li></ol>	<ol style="list-style-type: none"><li>1. Storm characteristics: precipitation, intensity, duration, magnitude and movement of storm.</li><li>2. Initial loss</li><li>3. Evapotranspiration</li></ol>



**Fig. 6.3** Role of Drainage Density on the Hydrograph



$$N = aA^{0.2}$$

$N$  is time in days

$A$  is the drainage area in  $\text{km}^2$

$$a = 0.8$$

**Fig. 6.5** Base Flow Separation Methods

## HYDROLOGICAL SYSTEM MODEL

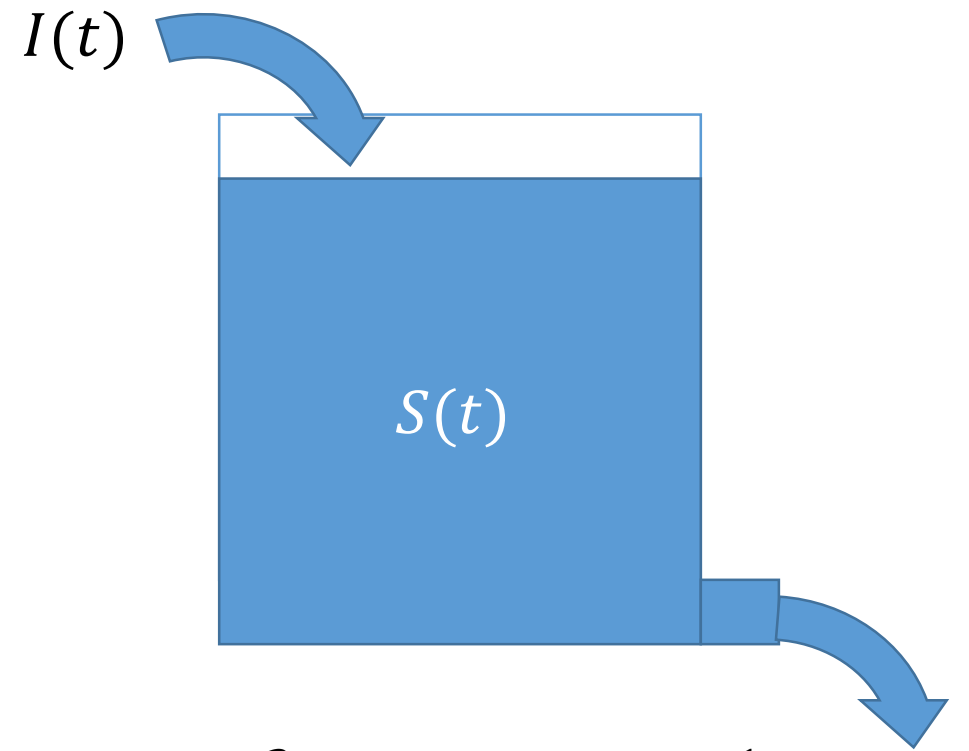
$$\frac{dS}{dt} = I - Q$$

$$S = f\left(I, \frac{dI}{dt}, \frac{d^2I}{dt^2}, \frac{d^3I}{dt^3}, \dots, Q, \frac{dQ}{dt}, \frac{d^2Q}{dt^2}, \frac{d^3Q}{dt^3}, \dots\right)$$

$$S = a_1 Q + a_2 \frac{dQ}{dt} + a_3 \frac{d^2 Q}{dt^2} + \dots + a_n \frac{d^{n-1} Q}{dt^{n-1}} + b_1 I + b_2 \frac{dI}{dt} + b_3 \frac{d^2 I}{dt^2} + \dots + b_m \frac{d^{m-1} I}{dt^{m-1}} \quad Q(t)$$

Differentiating

$$\frac{dS}{dt} = a_1 \frac{dQ}{dt} + a_2 \frac{d^2 Q}{dt^2} + a_3 \frac{d^3 Q}{dt^3} + \dots + a_n \frac{d^n Q}{dt^n} + b_1 \frac{dI}{dt} + b_2 \frac{d^2 I}{dt^2} + b_3 \frac{d^3 I}{dt^3} + \dots + b_m \frac{d^m I}{dt^m}$$





$$\frac{dS}{dt} = I - Q$$

$$a_1 \frac{dQ}{dt} + a_2 \frac{d^2 Q}{dt^2} + a_3 \frac{d^3 Q}{dt^3} +, \dots, a_n \frac{d^n Q}{dt^n} + b_1 \frac{dI}{dt} + b_2 \frac{d^2 I}{dt^2} + b_3 \frac{d^3 I}{dt^3} +, \dots, b_m \frac{d^m I}{dt^m} = I - Q$$

$$a_1 \frac{dQ}{dt} + a_2 \frac{d^2 Q}{dt^2} + a_3 \frac{d^3 Q}{dt^3} +, \dots, a_n \frac{d^n Q}{dt^n} + Q = I - b_1 \frac{dI}{dt} + b_2 \frac{d^2 I}{dt^2} + b_3 \frac{d^3 I}{dt^3} +, \dots, b_m \frac{d^m I}{dt^m}$$

$$N(D)Q = M(D)I$$

Where  $N(D) = a_1 \frac{d}{dt} + a_2 \frac{d^2}{dt^2} + a_3 \frac{d^3}{dt^3} +, \dots, a_n \frac{d^n}{dt^n} + 1$

Considering  $a_1 = k$  and all other coefficients zero

$$M(D) = -b_1 \frac{d}{dt} + b_2 \frac{d^2}{dt^2} + b_3 \frac{d^3}{dt^3} +, \dots, b_n \frac{d^n}{dt^n} + 1$$

$$k \frac{dQ}{dt} + Q = I$$

Solving for  $Q$   $Q = \frac{M(D)}{N(D)} I$

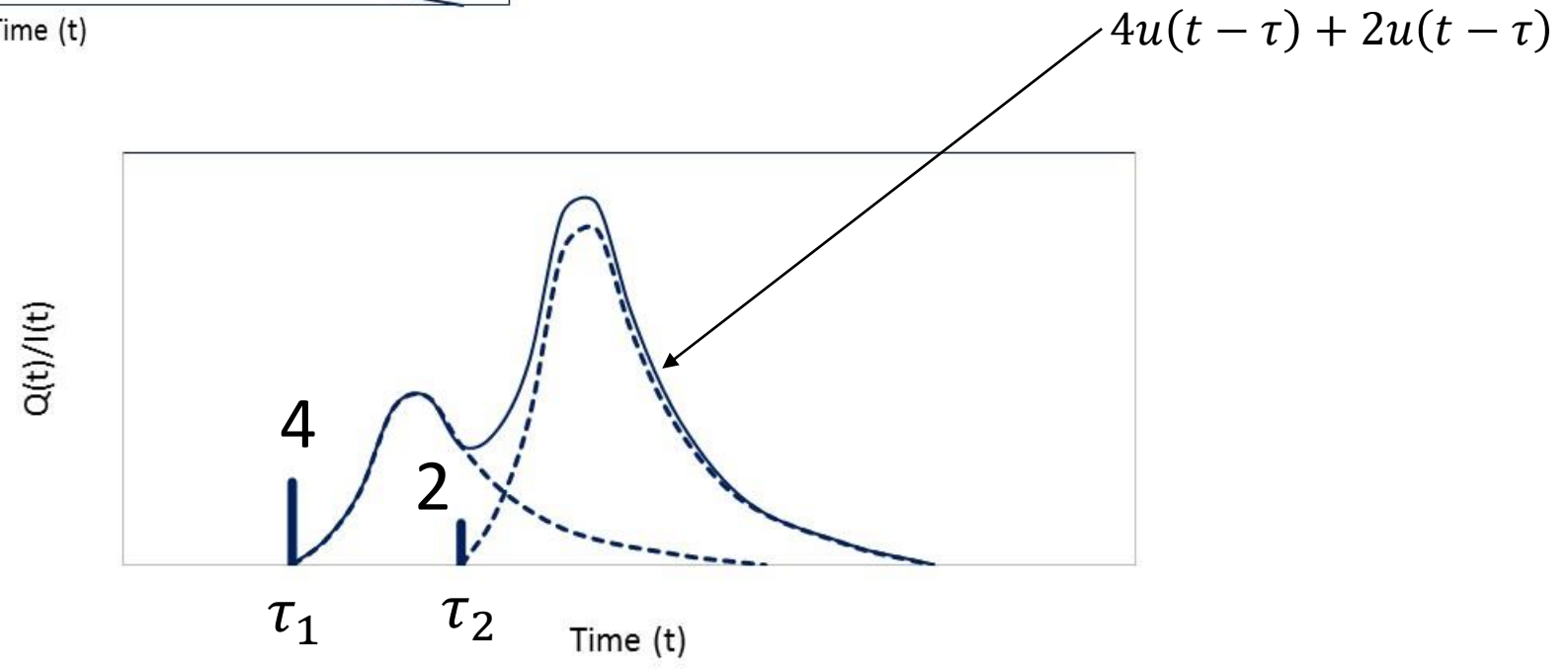
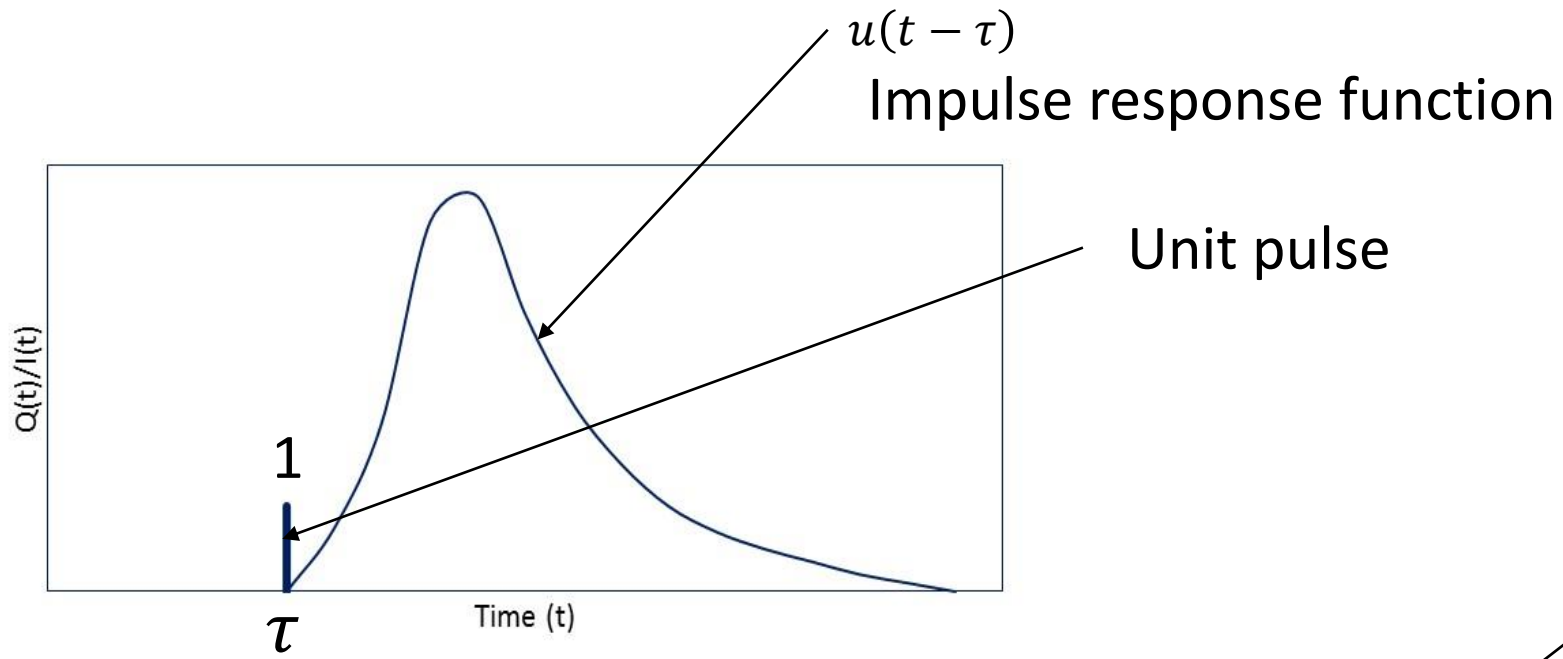
## Response function of linear system

Flow the two principle of linearity: Principle of proportionality and Superposition

**Principle of proportionality:** If a solution of  $f(x)$  is multiplied by a constant  $c$ , the resulting function  $cf(x)$  is also a solution.

**Superposition:** If two solutions  $f_1(x)$  and  $f_2(x)$ , the resulting function  $f_1(x) + f_2(x)$  is also a solution

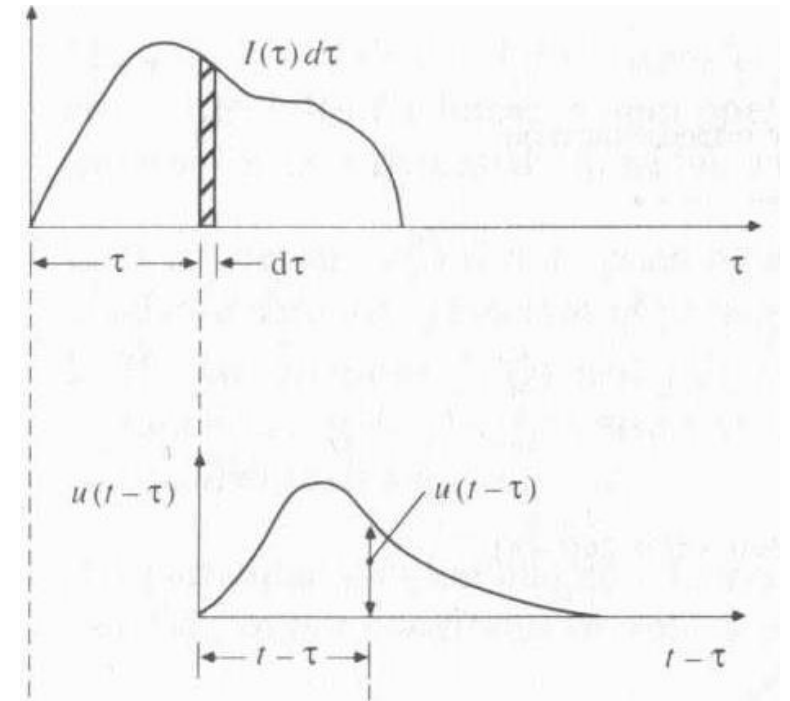
# Impulse Response function

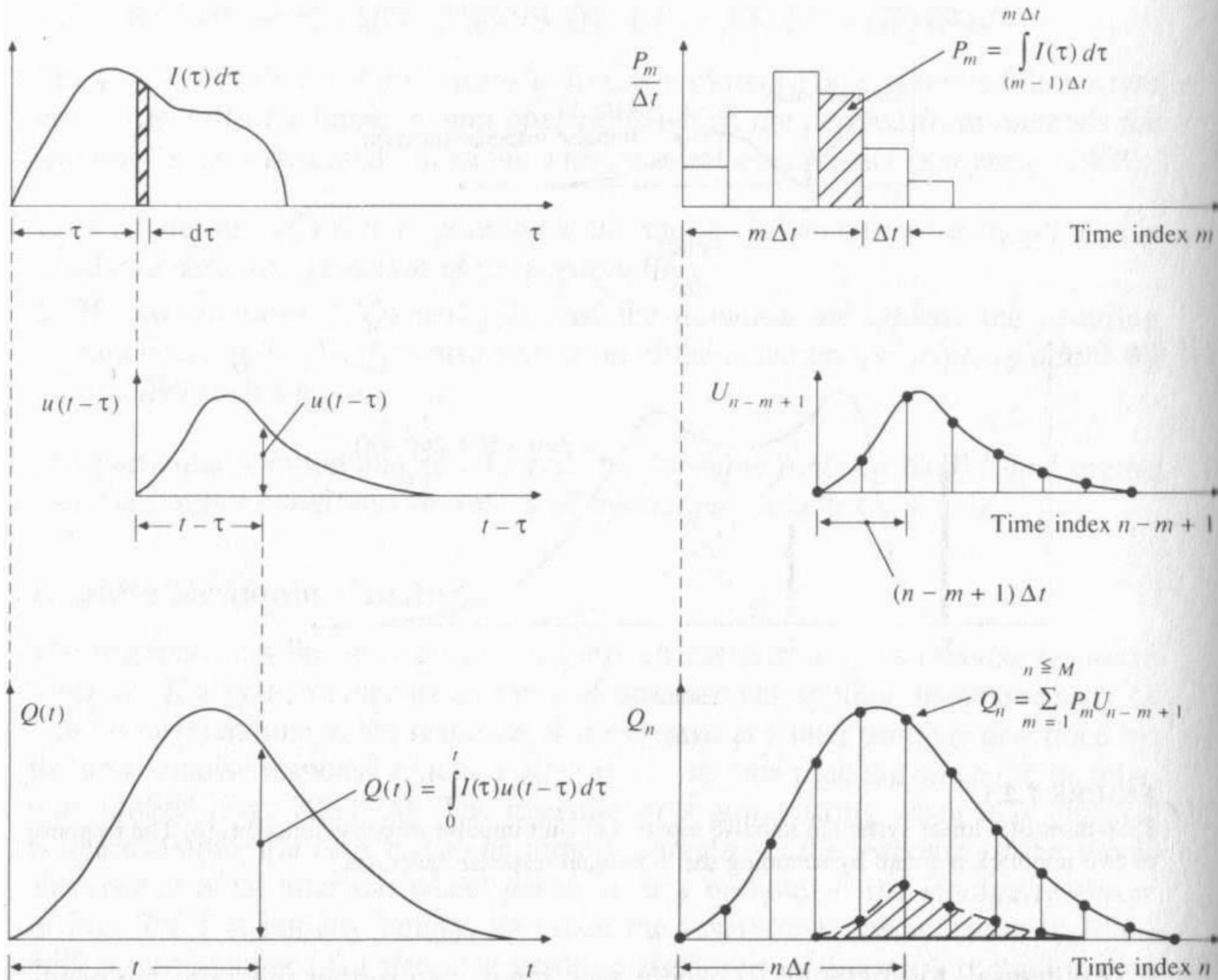


$$Q(t) = \int_0^t I(\tau)u(t - \tau)d\tau$$

This is known as convolution integral

This is the fundamental equation for solution of linear system on a continuous time scale





(a) Continuous time functions

(b) Discrete time functions



## Step response function

A unit step input is an input that goes from 0 to 1 at time 0 and continuous indefinitely at that rate thereafter

$$Q(t) = g(t) = \int_0^t I(\tau)u(t - \tau)d\tau \quad I(\tau) = 1$$

$$Q(t) = g(t) = \int_0^t u(t - \tau)d\tau$$

Substituting  $l = t - \tau$       $d\tau = -dl$

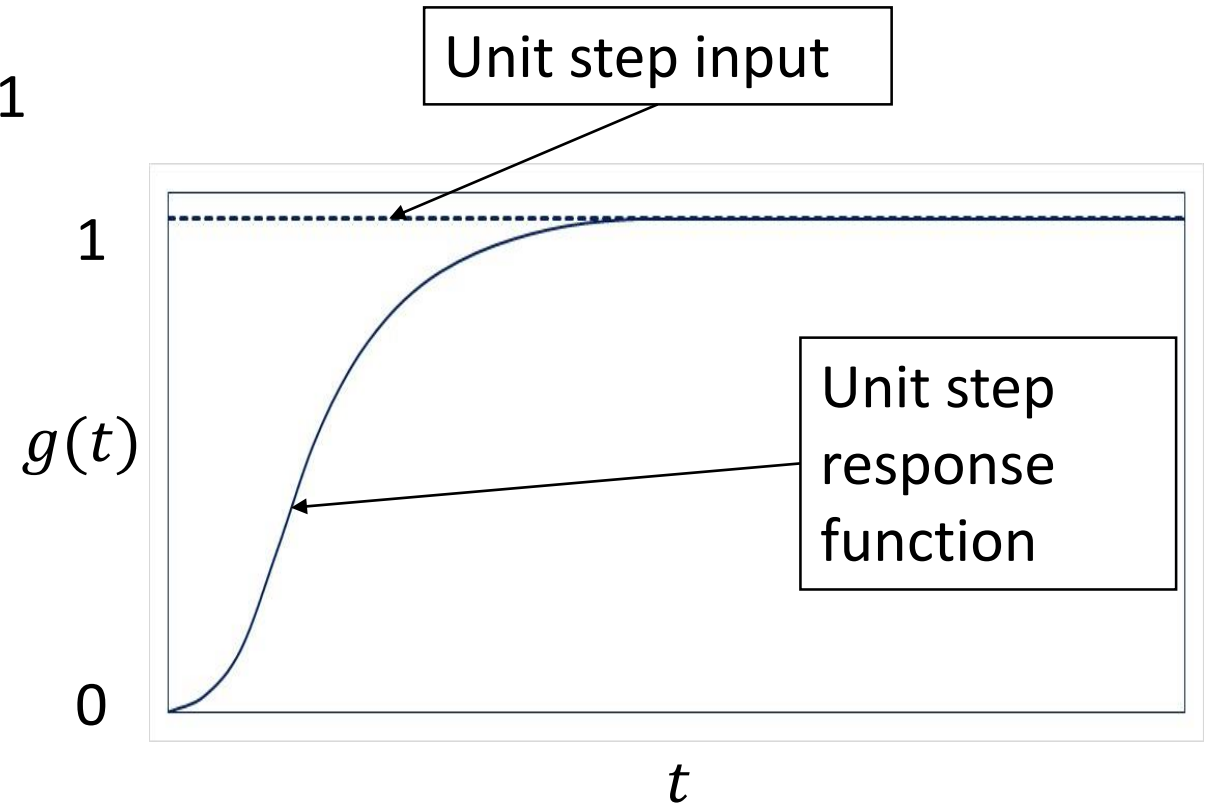
The limit  $\tau = t$ , becomes  $l = t - t = 0$

The limit  $\tau = 0$ , becomes  $l = t - 0 = t$

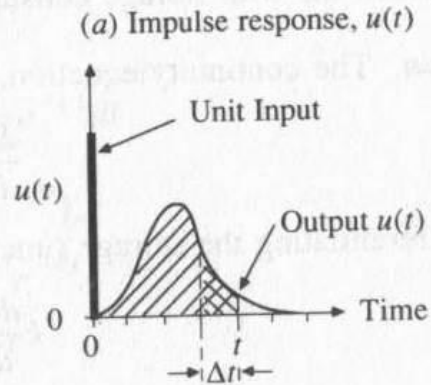
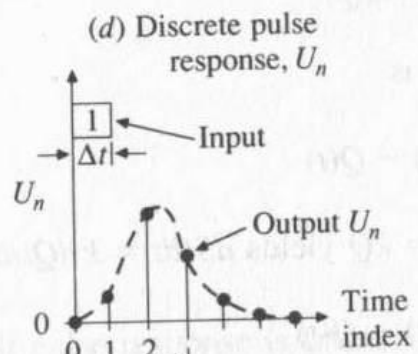
$$g(t) = - \int_t^0 u(l)dl$$

$$g(t) = \int_0^t u(l)dl$$

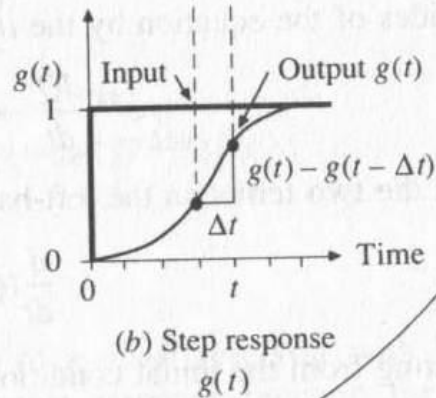
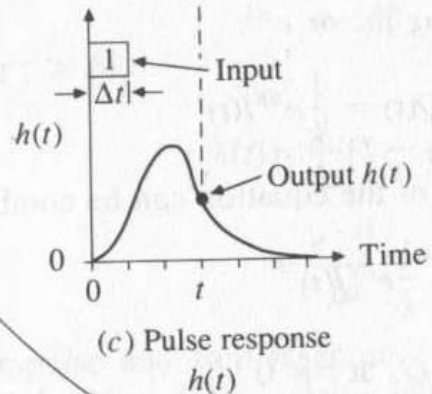
Thus the unit step response function at time  $t$  is the integral of the impulse response function up to the time



$$\lim_{\Delta t \rightarrow 0} h(t) = u(t)$$



$$U_n = h(n \Delta t)$$



$$g(t) = \int_0^t u(t) dt$$

$$h(t) = \frac{1}{\Delta t} [g(t) - g(t - \Delta t)]$$

## Pulse response function

A unit pulse input is an input of unit amount occurring in duration  $\Delta t$ . The rate is

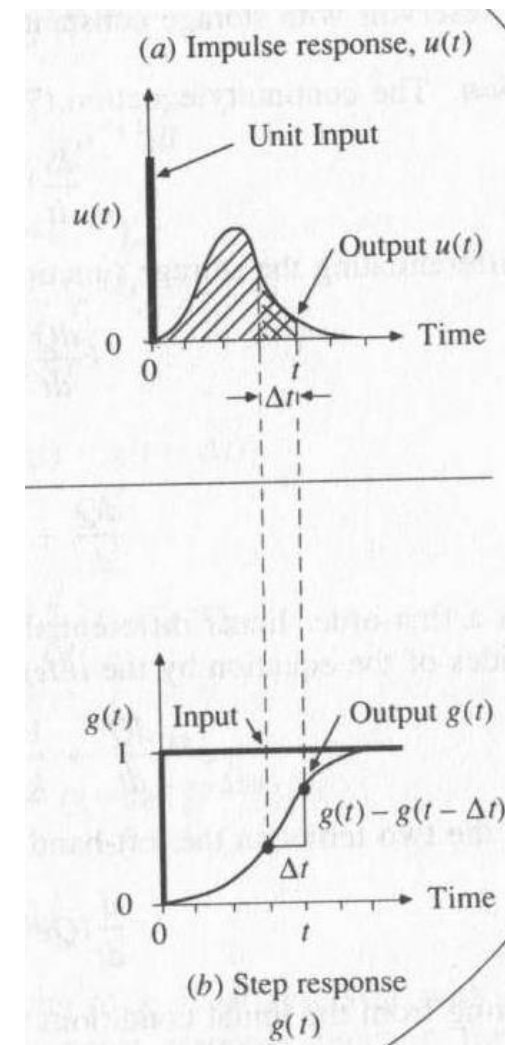
$I(\tau) = 1/\Delta t$ , between  $0 \leq \tau \leq \Delta t$  and 0 elsewhere

Response to a unit step input of rate  $1/\Delta t$  beginning at time 0 is  $(1/\Delta t)g(t)$

Response to a unit step input of rate  $1/\Delta t$  beginning at time  $\Delta t$  is  $(1/\Delta t)g(t - \Delta t)$

Unit pulse response function

$$h(t) = \frac{1}{\Delta t} (g(t) - g(t - \Delta t))$$



## Pulse response function

$$h(t) = \frac{1}{\Delta t} (g(t) - g(t - \Delta t))$$

$$= \frac{1}{\Delta t} \left( \int_0^t u(l) dl - \int_0^{t-\Delta t} u(l) dl \right)$$

$$= \frac{1}{\Delta t} \int_{t-\Delta t}^t u(l) dl$$

## Step response function

$$g(t) = \int_0^t u(l) dl$$

## Pulse response function

$$h(t) = \frac{1}{\Delta t} \int_{t-\Delta t}^t u(l) dl$$

## Unit Hydrograph

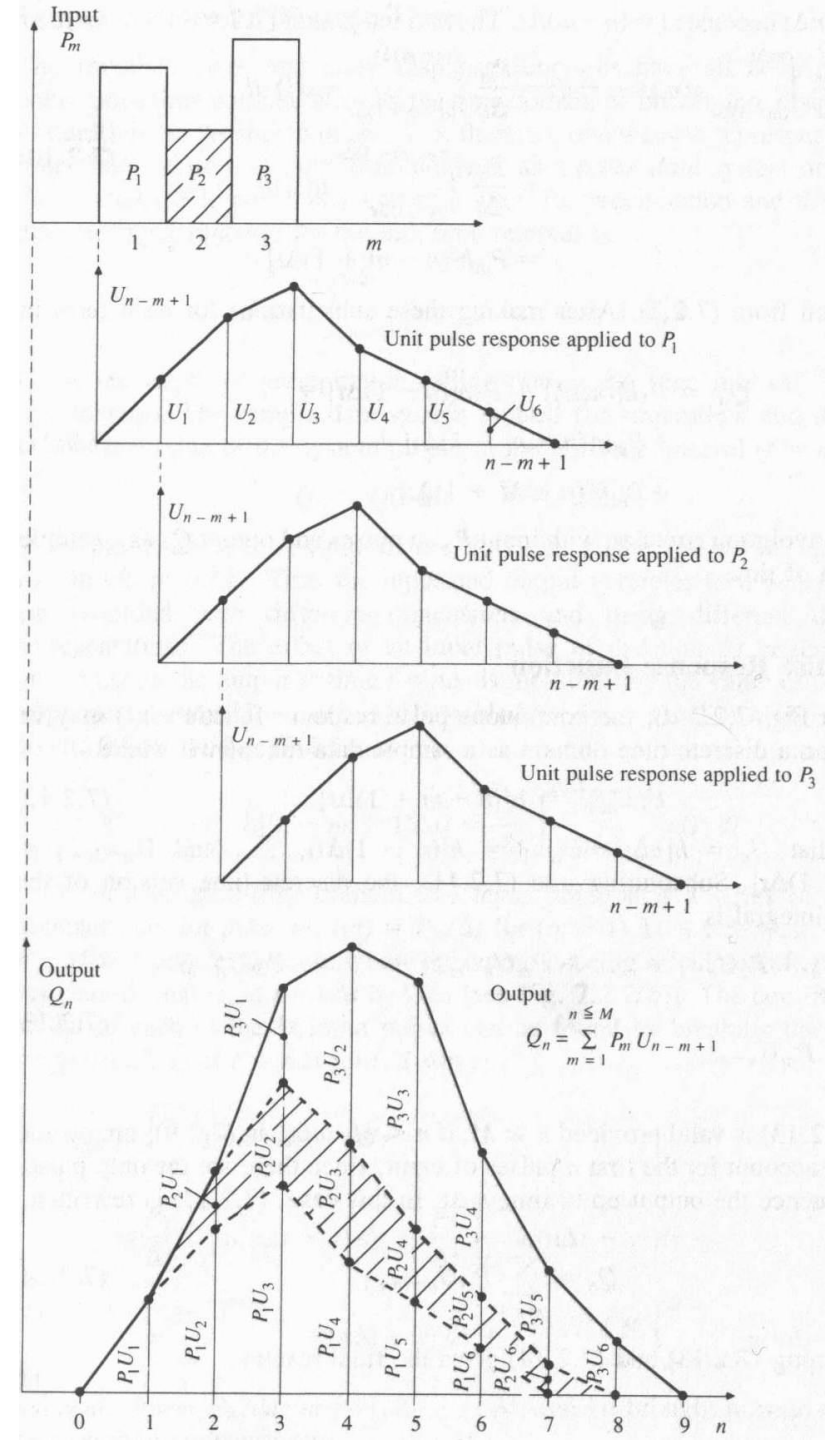
The unit hydrograph is the unit pulse response function of a linear hydrologic system

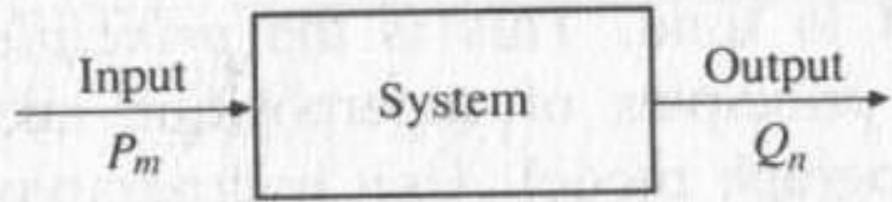
It was first proposed by Sherman (1932), the unit hydrograph of a watershed is defined as a direct runoff hydrograph resulting from 1 cm of excess rainfall generated uniformly over the drainage area.

- The excess rainfall has a constant intensity within the effective duration
- The excess rainfall is uniformly distributed throughout the whole drainage area
- The base time of the DRH resulting from an excess rainfall of given duration is constant
- The hydrograph resulting from a given excess rainfall reflects the unchanging characteristics of the watershed
- The ordinate of all DRH's of a common base time are directly proportional to the total amount of direct runoff represented by each hydrograph



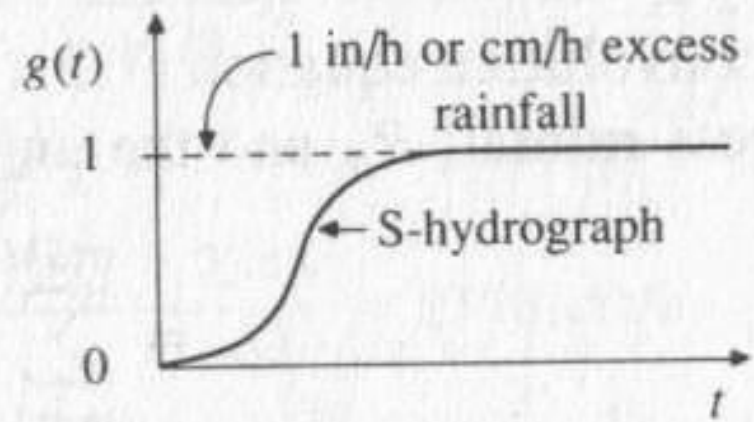
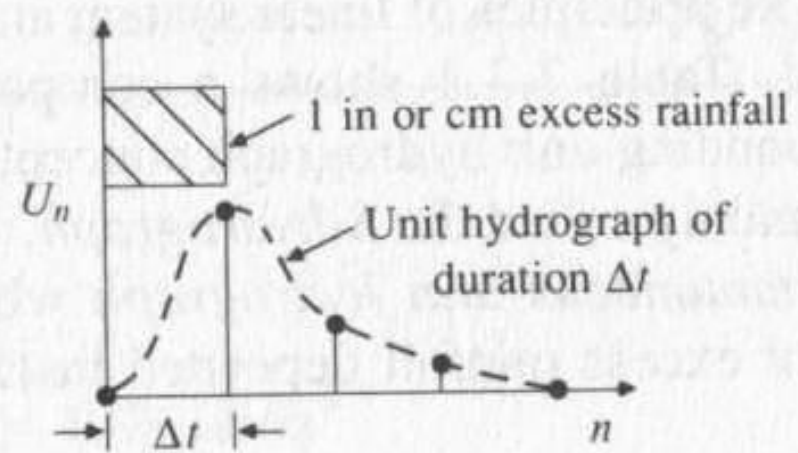
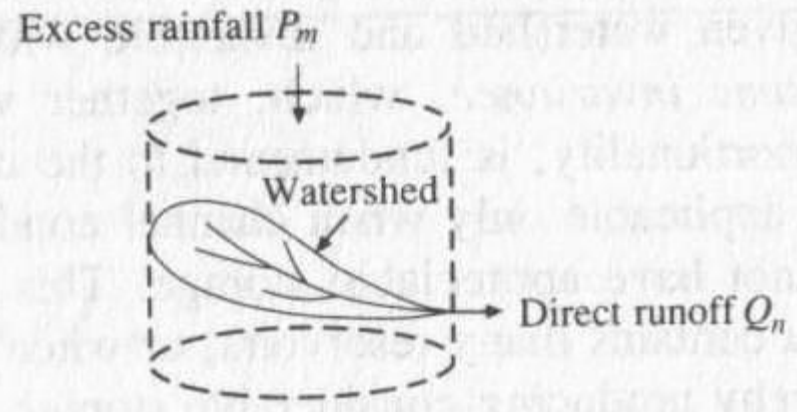
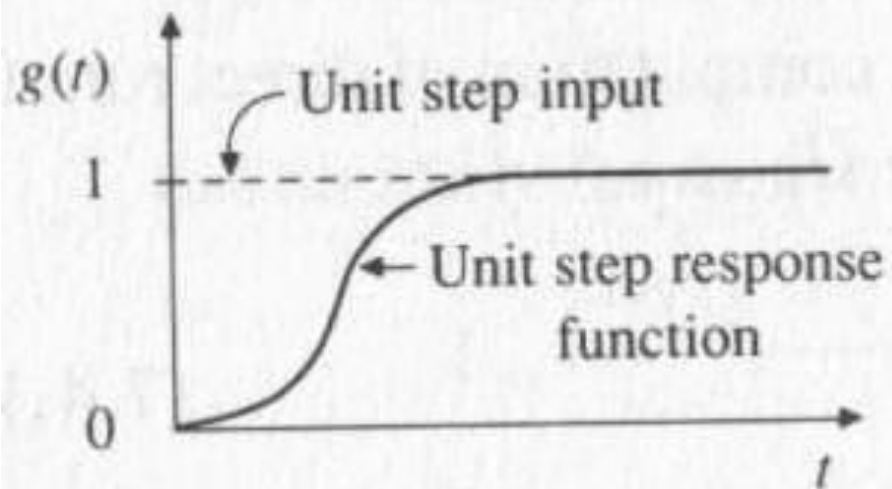
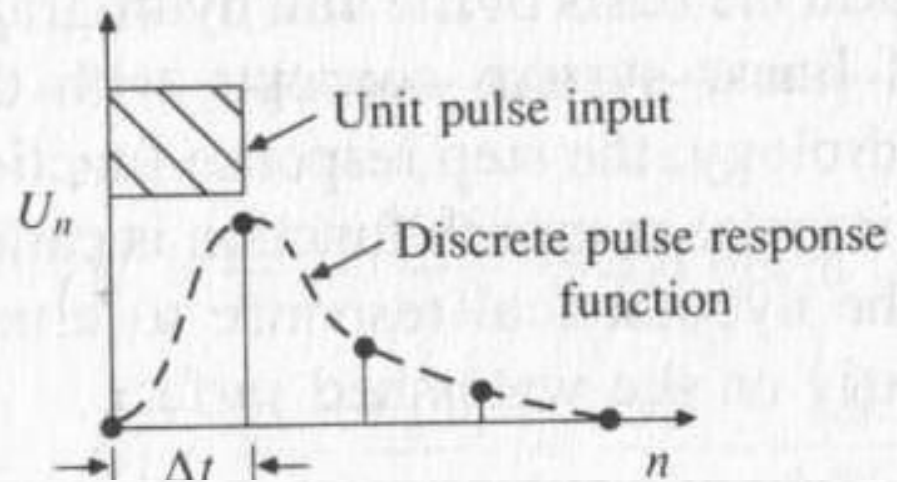
$$Q_n = \sum_{m=1}^{n \leq M} P_m U_{n-m+1}$$

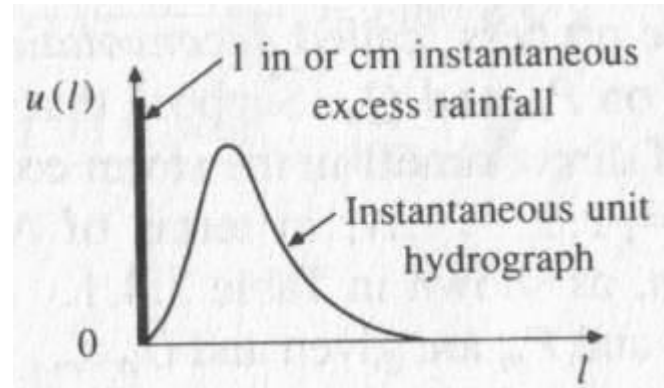
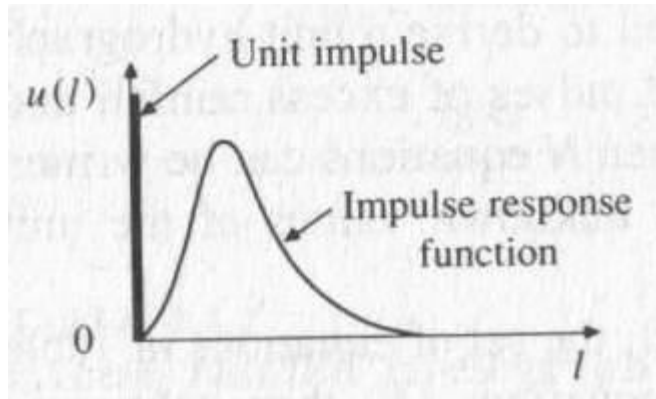




$$n \leq M$$

$$Q_n = \sum_{m=1} P_m U_{n-m+1}$$

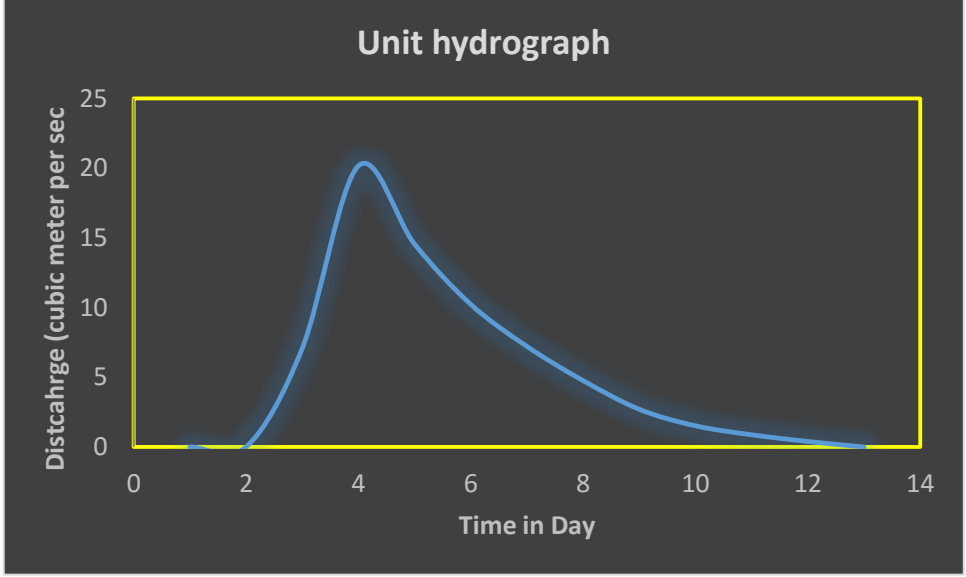




# Estimate Unit hydrograph of the catchment

Time (Days)	Total Runoff (m3/s)	Base Flow (m3/s)	Direct runoff (m3/s)	Unit Hydrograph (m3/s) per mm
1	168	168	0	0
2	160	160	0	0
3	500	160	340	7
4	1130	160	970	20
5	860	160	700	15
6	650	158	492	10
7	500	153	347	7
8	380	151	229	5
9	280	150	130	3
10	220	147	73	2
11	185	143	42	1
12	160	141	19	0
13	140	140	0	0

Catchment area is 6500 sq. km



# UNIT HYDROGRAPH OF DIFFERENT DURATION

$$h(t) = \frac{1}{\Delta t} [g(t) - g(t - \Delta t)]$$

$$h(t - \Delta t) = \frac{1}{\Delta t} [g(t - \Delta t) - g(t - 2\Delta t)]$$

$$h(t - 2\Delta t) = \frac{1}{\Delta t} [g(t - 2\Delta t) - g(t - 3\Delta t)]$$

$$h(t - 3\Delta t) = \frac{1}{\Delta t} [g(t - 3\Delta t) - g(t - 4\Delta t)]$$

⋮

$$g(t) = \Delta t [h(t) + h(t - \Delta t) + h(t - 2\Delta t) + \dots]$$

$$g(t) = g(t - \Delta t')$$

$$h'(t) = \frac{1}{\Delta t'} [g(t) - g(t - \Delta t')]$$

