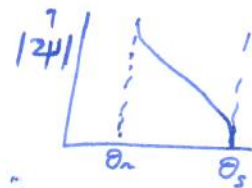


In the last class we were dealing with unsaturated flow.

\* We have seen that the moisture content and suction head are related that can be expressed by empirical relations described by many scientists

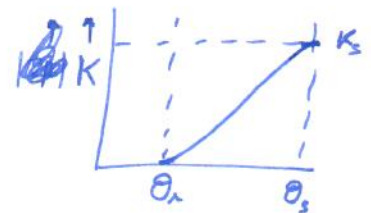
- viz: Gardner (1958), Brooks and Corey (1964),

van Genuchten (1980), etc.



\* In a similar tone we can also related hydraulic conductivity with water content and subsequently with suction head.

\* The one dimensional Richards equation



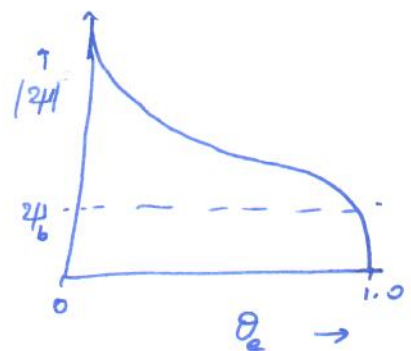
$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial z} \left( D \frac{\partial \theta}{\partial z} + K \right)$$

can be solved using these constitutive relationships.

For example

1) The Brooks-Corey relationship between moisture content and suction head is

$$\theta_e = \frac{\theta - \theta_n}{\theta_s - \theta_n} = \left( \frac{h_b}{h} \right)^{\lambda}$$

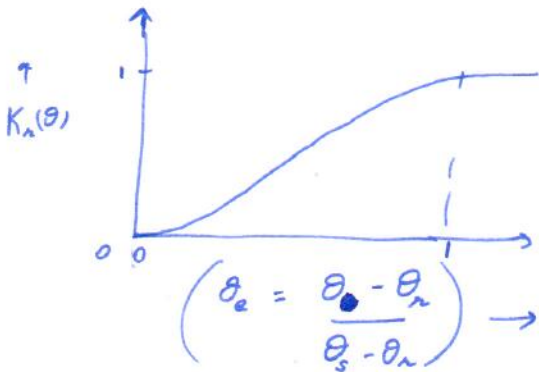


(2)

Brooks and Lory (1964) have also given relationship between  $K$  and  $\theta$ .

$$\frac{K(\theta)}{K_s} = K_n(\theta) = (\theta_e)^n$$

where  $n = 3 + \frac{2}{\lambda}$



2) Similarly van Genuchten have given relationship between  $\theta$  and  $w$  as such:

$$\theta_e = \frac{\theta - \theta_n}{\theta_s - \theta_n} = \left[ 1 + (\alpha |w|)^n \right]^m$$

where  $\alpha \rightarrow$  parameter related to air entry pressure [ $L^{-1}$ ]

$n \rightarrow$  constant

$m \rightarrow$  constant

$$K_n(\theta) =$$

## SOLUTE TRANSPORT

In groundwater hydrology, we have seen about flow in aquifers, well hydraulics, briefly about unsaturated zone hydrology, etc.

\* An important subject for a groundwater hydrologist is contaminant transport modeling.

⇒ The groundwater ~~is~~ <sup>beds</sup> ~~sources~~ are highly vulnerable to contamination from various sources like:

\* Septic tanks, oil disposal, landfill leaching, mine water and drainages, tannery, pesticides, etc.

⇒ We know that the speed of groundwater is very ~~slow~~ compared to surface water bodies. Therefore, if any contaminant enters into a groundwater system, it may remain there for a long time. So it may not be possible to wash away the contaminants at rapid speed.

⇒ Scientific studies of contaminant movement through subsurface - including aquifers and unsaturated zones are essential to understand the impacts and extents of contamination at any region.

(4)

Recalling our earlier chapters - ~~Obj~~: Chapter 4 where we dealt with continuity and momentum equations for multi-specie fluid, we can employ same principles here.

i.e. ~~Assumption~~

⇒ A porous medium subjected to a certain contamination may be considered as a multi-specie fluid.

∴ Based on mass-averaged velocity  $\vec{V}^*$ , you can have diffusive and dispersive components for ~~contaminants~~ the respective contaminant specie.

⇒ Modeling such contaminant transport are essential for groundwater contamination mitigation

⇒ The processes that involve contaminant transport are:

- Advection
- Diffusion
- Dispersion

In contaminant transport studies: the

Molecular diffusion

⊙ The solute or contaminant moves from higher concentration to lower concentration and is called diffusion.



(5)

The term concentration suggest about the strength or portion of contaminant in the water. The units may be  $\rightarrow$   $\frac{\text{mass of contaminant}}{\text{Unit volume of water}}$

or g mass of contaminant per unit mass of water, etc.

$\Rightarrow$  The term diffusion suggest about spreading of the contaminants.



Contaminant initially concentrated at a location



After some time the contaminant intensity decreased but spread more in the domain

$\Rightarrow$  The laws of diffusion are very much applicable to the situation here:

Fick's first law:

If 'F' is mass flux of solute (or contaminant) per unit time per unit area (i.e.  $ML^{-2}T^{-1}$  like  $\frac{kg}{m^2s}$  etc.)

$$F \propto -\frac{\partial C}{\partial x}$$

$$F = -D_d \frac{\partial C}{\partial x}$$

(See the analogy w.r.t. Darcy's law)

where  $D_d \rightarrow$  diffusion coefficient  
 $\frac{\partial C}{\partial x} \rightarrow$  gradient of concentration in space.

(6)

Fick's second law:



Considering a one dimensional control volume shown above, let the mass come into CV from left and goes out through right

$$\begin{aligned} \text{Let mass flux that comes in} &= F \\ \text{Mass flux that goes out} &= F + \frac{\partial F}{\partial x} \Delta x \end{aligned}$$

$$\begin{aligned} \text{Mass entering the control volume} &= F \Delta A \\ \text{per unit time} \end{aligned}$$

$$\begin{aligned} \text{Mass leaving control volume} &= \left( F + \frac{\partial F}{\partial x} \Delta x \right) \Delta A \\ \text{per unit time} \end{aligned}$$

$\therefore$  Net mass Using Reynolds Transport Theorem for mass conservation of solute

$$0 = \frac{\partial}{\partial t} \iiint_V \rho s \, dV + \iint_{\omega} \rho s (\vec{V} \cdot \hat{n}) \, dA$$

$$\begin{aligned} \iint_{\omega} \rho s (\vec{V} \cdot \hat{n}) \, dA &\rightarrow \text{Net mass outflow per unit time} \\ &= \frac{\partial F}{\partial x} \Delta x \Delta A \quad \left( \text{for the case of diffusion here} \right) \end{aligned}$$

$$\begin{aligned} \frac{\partial}{\partial t} \iiint_V \rho s \, dV &\rightarrow \text{Change in mass stored of contaminant} \\ &\text{stored in CV.} \\ &= \frac{\partial}{\partial t} (C \Delta x \Delta A) \end{aligned}$$

$$\therefore \frac{\partial C}{\partial t} \Delta x \Delta A = - \frac{\partial F}{\partial x} \Delta x \Delta A$$

(7)

$$u \quad \frac{\partial C}{\partial t} = - \frac{\partial F}{\partial x}$$

Recall  $F = - D_d \frac{\partial C}{\partial x}$

$$\therefore \frac{\partial C}{\partial t} = - \frac{\partial}{\partial x} \left( - D_d \frac{\partial C}{\partial x} \right)$$

$$u \quad \frac{\partial C}{\partial t} = \frac{\partial}{\partial x} \left( D_d \frac{\partial C}{\partial x} \right)$$

If diffusivity  $D_d$  is independent of space,

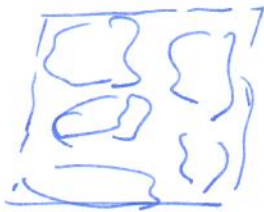
$$\frac{\partial C}{\partial t} = D_d \frac{\partial^2 C}{\partial x^2}$$

(Fick's second law)

### Mechanical Dispersion

The contaminant transport in addition to molecular diffusion may occur due to mechanical dispersion.

→ As the term suggests this is caused by mechanical process. Mixing of contaminant occurs due to difference in speeds of movement of contaminants



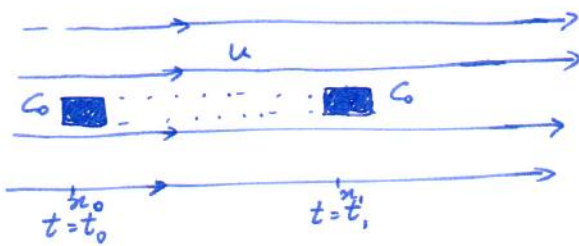
Before discussing mechanical dispersion, we should discuss first about contaminant transport process

(8)

through advection.

The main movement due to bulk motion of fluid describes the advection.

i.e. In a uniform flow field



In the figure description given here, the contaminant with concentration  $C_0$  is applied at  $x = x_0$  in uniform flow field.

If the concentration of the contaminant

is  $C_0$  at time  $t = t_1$  at  $x = x_1$ , with same dimension, this means that

the contaminant has not spread and the movement of contaminant occurred purely due to the bulk movement of water.

$\Rightarrow$  If there is difference in such bulk velocities or advective velocities, then a gradient exist that can cause dispersive contaminant mass flux.

$$F_m = -D_m \frac{\partial C}{\partial x} \quad \text{and} \quad \frac{\partial C}{\partial t} = \frac{\partial}{\partial x} \left( -D_m \frac{\partial C}{\partial x} \right)$$

$$\text{where } D_m = \alpha_L V_L$$

$\alpha_L \rightarrow$  longitudinal mechanical dispersivity

$V_L \rightarrow$  pore velocity