

**NRCS METHOD TO DETERMINE SURFACE RUNOFF:**

In the last class, we introduced to you the NRCS method.

Recalling the terms,

$P$  = the depth of rainfall.

$P_e$  = Depth of excess rainfall

$F_a$  = Depth of water retained in watershed.

$I_a$  = Depth of water for initial obstruction

$S$  = potential maximum retention for the watershed,

Then  $P - I_a$  = potential runoff possible

The SCS method suggest that the ratio of two actuals (i.e.  $F_a$  and  $P_e$ ) to the two potentials (i.e.  $S$  and  $P - I_a$ ) are equal

$$\therefore \frac{F_a}{P_e} = \frac{S}{P - I_a}$$

$$\text{or } \frac{F_a}{S} = \frac{P_e}{P - I_a}$$

$$P = P_e + I_a + F_a$$

Rearranging and solving, we get

$$P_e = \frac{(P - I_a)^2}{P - I_a + S}$$

This is the basic equation for computing the depth of excess rainfall or direct runoff from a storm using NRCS method

- It is seen that through many observations, empirically

$$I_a = 0.2S$$

$$\therefore P_e = \frac{(P - 0.2S)^2}{P + 0.8S}$$

Further empirical studies suggest that the potential maximum retention  $S$  in inches is

$$S = \frac{1000}{CN} - 10$$

Where  $CN$  is runoff curve number, which is a function of land use, antecedent soil moisture etc.

- Curve number,  $CN$  is dimensionless  $0 \leq CN \leq 100$
- For impervious surface and/ or water surface ,  $CN = 100$
- For natural surface  $CN < 100$

## ANTECEDENT MOISTURE CONDITION

The curve number obtained through various experiment are plotted for  $P_e$  (in inches) verses cumulative rainfall  $P$  (in inches)

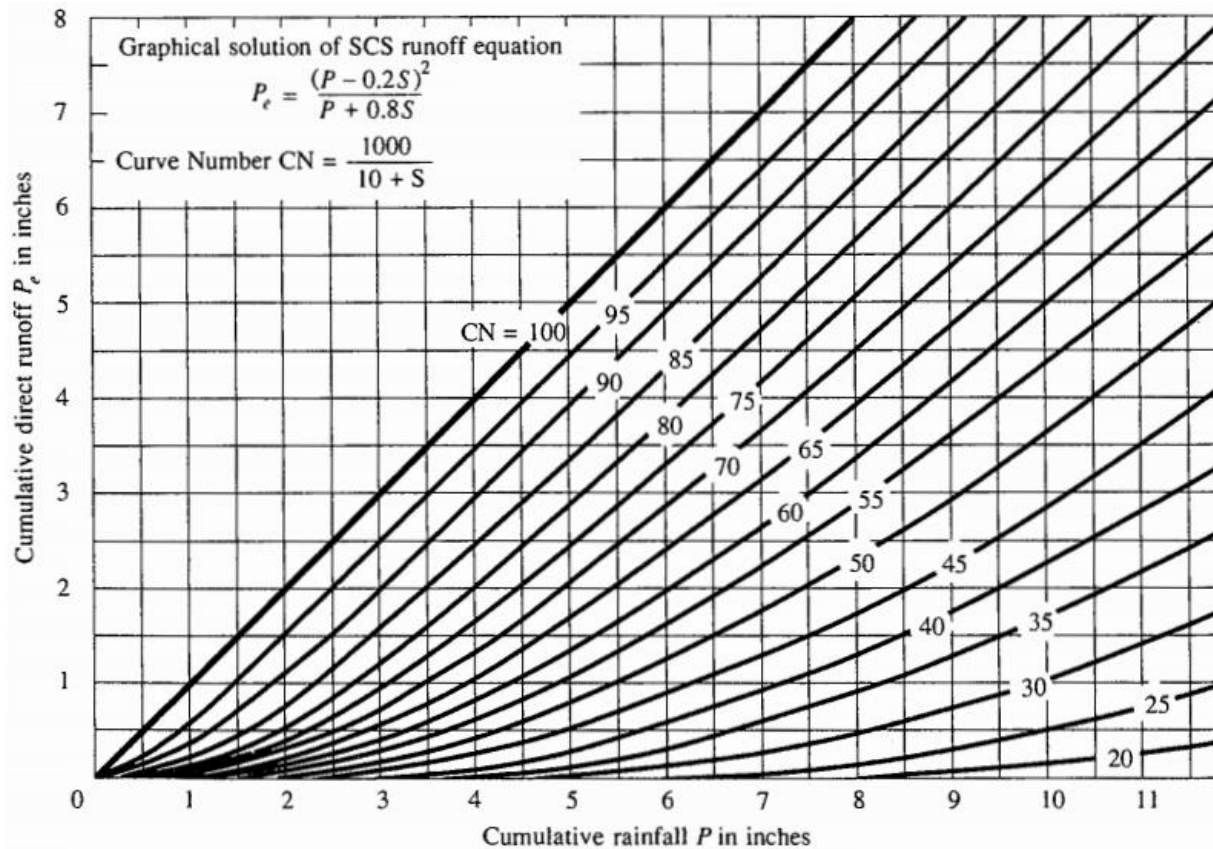


Figure: Solution of NRCS Runoff equation (provided by SCS)  
(The snapshot image copied from the course text book Chow et al. (1988))

- This curve number is for antecedent moisture condition II (AMC II)
- For dry condition, you can use AMC I and for wet conditions you can use AMC III

$$CN(I) = \frac{4.2CN(II)}{10 - 0.058CN(II)}$$

$$CN(III) = \frac{23 CN(II)}{10 + 0.13CN(II)}$$

- So in graphs and tables you are actually provided with curve number for AMC II.
- Empirically, it is observed that the initial abstraction  $I_a = 0.2S$  for many small watersheds in USA.

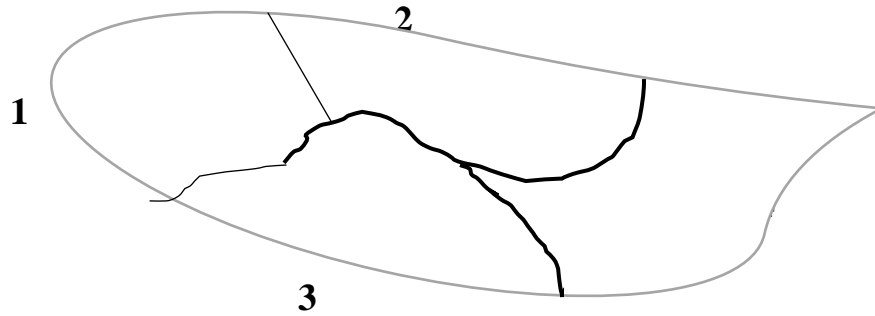
**Table: Runoff curve numbers for selected agricultural, suburban, and urban land uses (Antecedent Moisture Condition II and  $I_a = 0.2S$ )**

Land Use Description	Hydrologic Soil Group			
	A	B	C	D
Cultivated land <sup>1</sup> : without conservation treatment	72	81	88	91
with conservation treatment	62	71	78	81
Pasture or range land: poor condition	68	79	86	89
good condition	39	61	74	80
Meadow: good condition	30	58	71	78
Wood or forest land: thin stand, poor cover, no mulch	45	66	77	83
good cover <sup>2</sup>	25	55	70	77
Open Spaces, lawns, parks, golf courses, cemeteries, etc.				
good condition: grass cover on 75% or more of the area	39	61	74	80
fair condition: grass cover on 50% to 75% of the area	49	69	79	84
Commercial and business areas (85% impervious)	89	92	94	95
Industrial districts (72% impervious)	81	88	91	93
Residential <sup>3</sup> :				
Average lot size	Average % impervious <sup>4</sup>			
1/8 acre or less	65	77	85	90
1/4 acre	38	61	75	83
1/3 acre	30	57	72	81
1/2 acre	25	54	70	80
1 acre	20	51	68	79
Paved parking lots, roofs, driveways, etc. <sup>5</sup>	98	98	98	98
Streets and roads:				
paved with curbs and storm sewers <sup>5</sup>	98	98	98	98
gravel	76	85	89	91
dirt	72	82	87	89

(The table is taken as snapshot from the course text book Chow et al. (1988))

For a watershed if the areas are divided based on curve numbers, a weighted curve number can be evaluated.

Weighted curve number,



$$CN = \frac{1}{100} [L_1 * CN_1 + L_2 * CN_2 + \dots]$$

$L_1, L_2, L_3, \dots$  percentage of land use

- The soils are classified by SCS as:
- Group A that may consist of Deep sand, Deep loess, aggregated silts, etc.
- Group B that consist of Shallow loess, sandy loam, etc.
- Group C that consist of clay loams, shallow sandy loam, soils low in organic content, etc.
- Group D consist of soils that swell significantly, heavy plastic clays, etc.

**EXAMPLE:**

Determine weighted curve number for a watershed with 40% residential, 25% open space (good condition), 20% commercial and business (i.e. 85% impervious) 15% industrial (i.e. 72% impervious) the soil groups are C, D, C and D.

From the runoff curve number table provided in the text book:

For residential area with soil group C and ¼ is CN=83

For open space, for soil D, the CN=80

For commercial place soil C, the CN =94

For industrial location in soil D, the CN=93

Therefore weighted curve number,

$$\begin{aligned} \text{CN} &= 0.40 \times 83 + 0.25 \times 80 + 0.20 \times 94 + 0.15 \times 93 \\ &= 86 \end{aligned}$$

To get runoff volume for a 6 inch rainfall:

$$\begin{aligned} P_e &= \frac{(P - 0.2 S)^2}{P + 0.8 S} \\ S &= \frac{1000}{86} - 10 \\ &= 1.63 \text{ inches} \\ P_e &= \frac{(6 - 0.2 * 1.63)^2}{6 + 0.8 * 1.63} \\ &= 4.41 \text{ inch} \end{aligned}$$

## To evaluate Abstractions using Infiltration equations

- Recall, we had studied Green-Ampt equation; Horton's equation; etc. to find the rate of infiltration.
- Using these equations also, we can determine the abstractions (assuming that other abstractions like retention storage; trapping; etc are negligible)
- If we use, Green-Ampt equation, then we need to determine the time required for ponding.
- You have to estimate, what will be the time required for ponding ( $t_p$ )?
- As you are aware, till ponding occurs, the infiltration rate and rainfall intensity will be same.
- Substitute these facts in non-linear cumulative infiltration equation of Green-Ampt and subsequently determine the ponding time.
- Once, the infiltration quantity is identified, the surface runoff can be estimated by deducting the abstractions from the total rainfall.