



UNIVERSITÀ
DEGLI STUDI
DEL MOLISE

Invited Talk
Universita degli Studi del Molise
Campobasso, Italy

Geotechnical Practices and Applications for Infrastructure Development



Dr. Arindam Dey

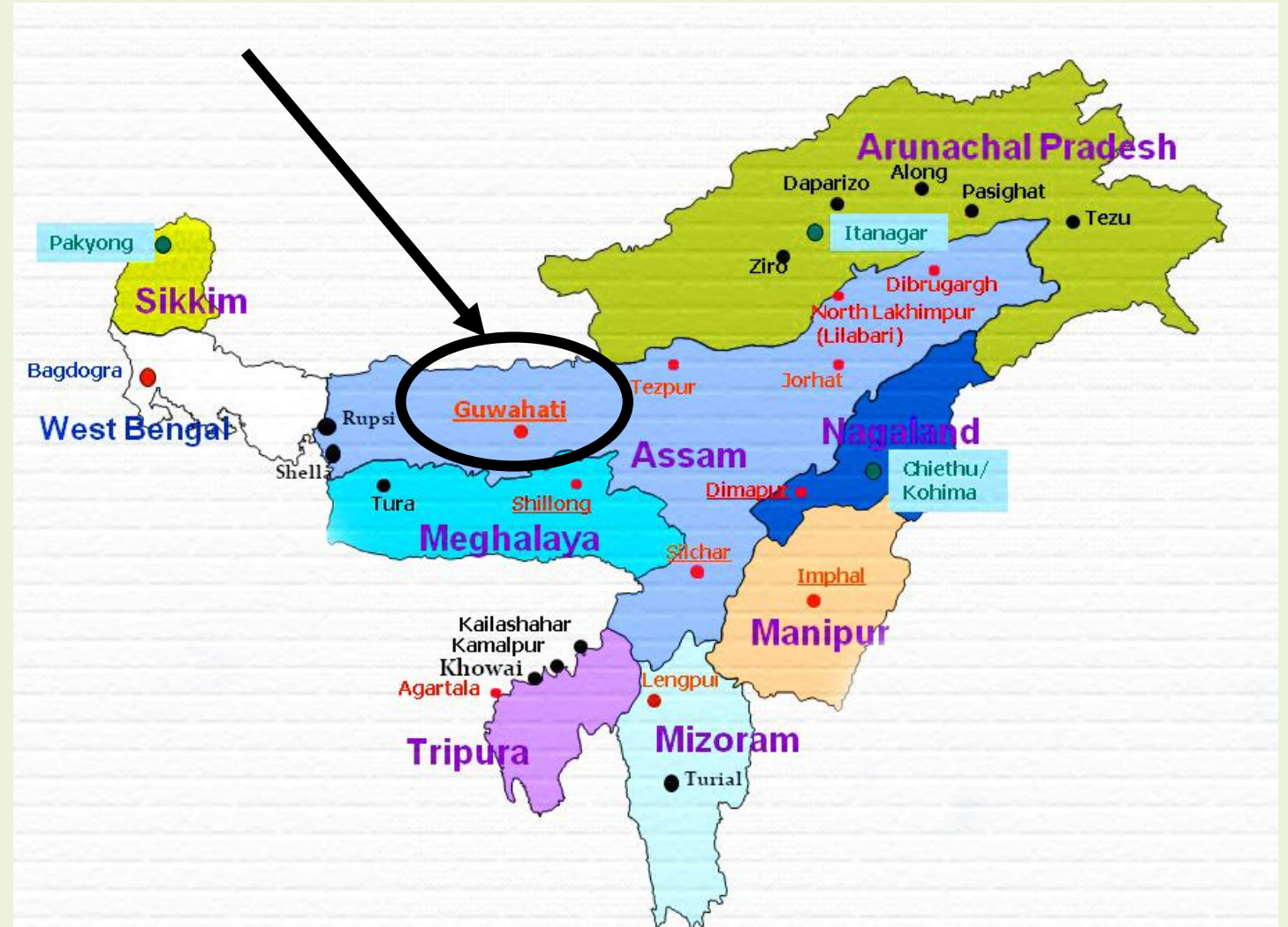
Associate Professor

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Center for Disaster Management and Research (CDMR)

Indian Institute of Technology Guwahati

Geographical Location of IIT Guwahati



Some Glimpses of the IIT Guwahati Campus



AD Research Group

- Students in the group (until date) - 159
 - ❖ Doctoral Research Students (23): Completed – 8, Ongoing – 15
 - ❖ M.Tech./MS(R) Students (34): Completed – 30, Ongoing – 4
 - ❖ B.Tech. Students (26)
 - ❖ Intern Students (47)
 - ❖ Credit Seminar Students (29)
- Research Publications (until date) - 269
 - ❖ Books: 2; Book Sections: 32; ASCE GSP: 3
 - ❖ International Journals: 51; National Journals: 15
 - ❖ ASCE Specialty Conference: 1; International Conference: 46; National Conference: 85
 - ❖ International Symposium: 7; National Symposium: 3
 - ❖ International Workshop: 3; National Workshop: 7
 - ❖ National Seminar: 8; Technical Reports: 6

Infrastructure and Development

- **Infrastructures** are the fundamental facilities and systems serving a country, city, or other area, including the services and facilities necessary for its economy to function.
 - ❖ **Infrastructures** are the physical components of interrelated systems providing commodities and services essential to enable, sustain, or enhance societal living conditions



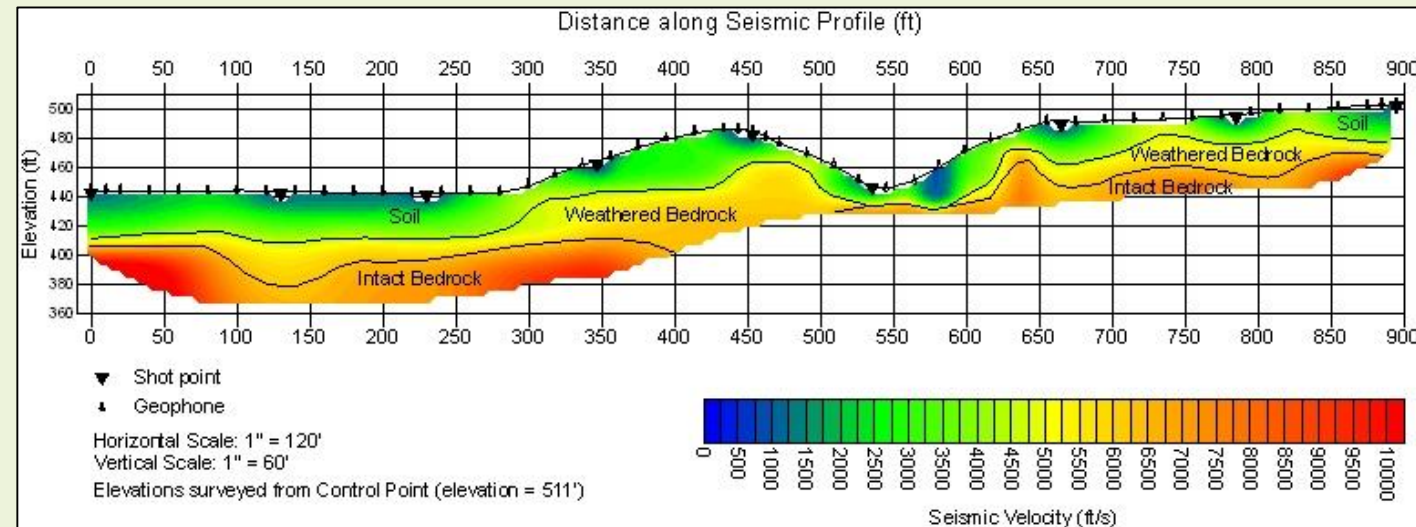
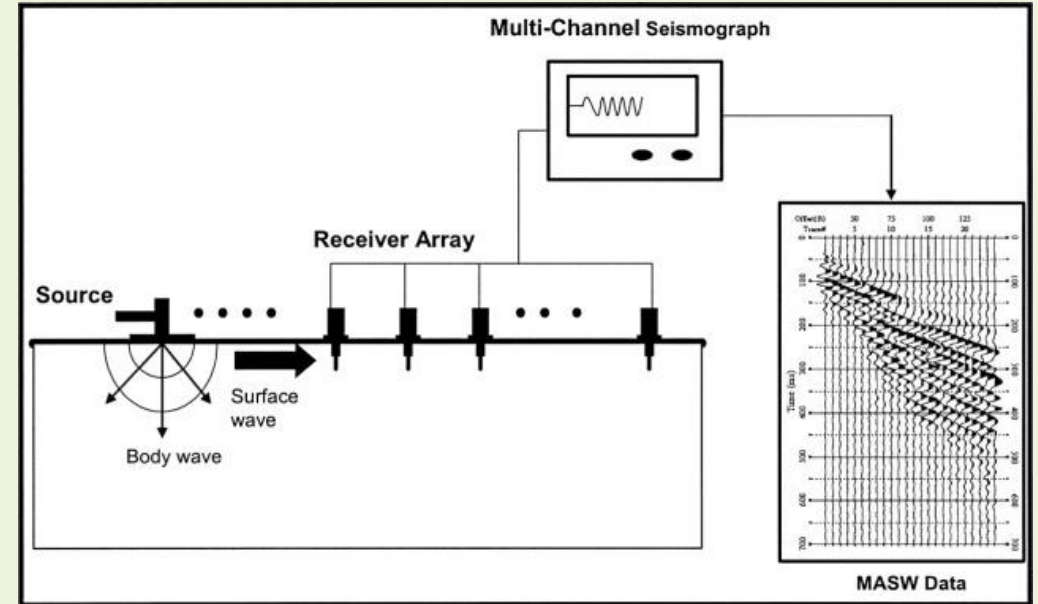
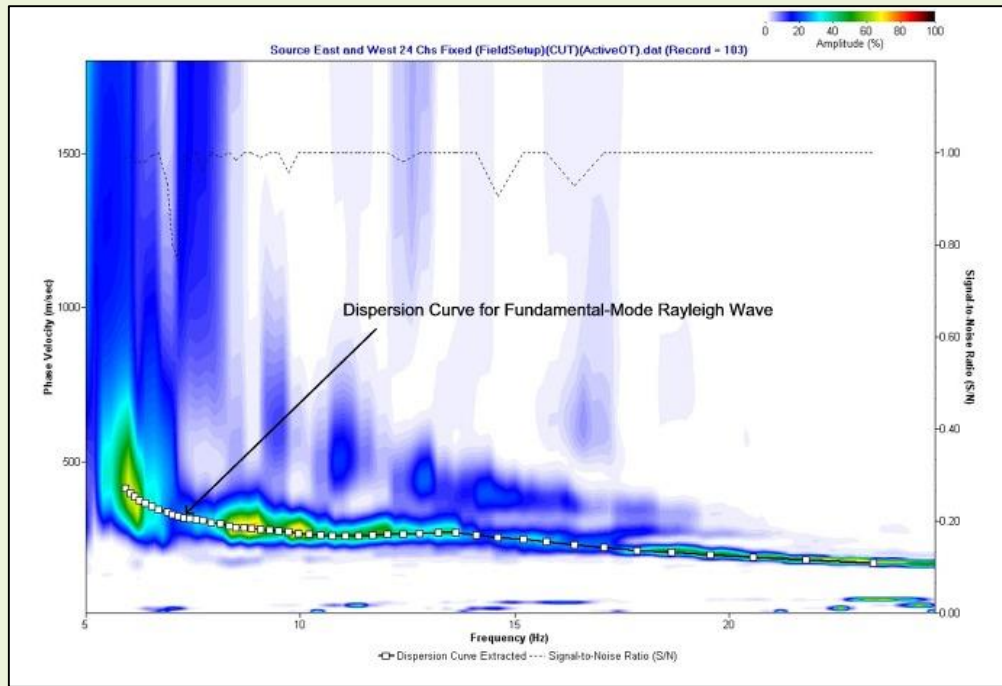
Geotechnical Engineering for Infrastructure Development

- Vistas of geotechnical engineering interrelated with infrastructure development
 - ❖ Geotechnical and Geophysical Investigations
 - ❖ Shallow, Deep and Hybrid foundations on horizontal grounds and slopes
 - ❖ Ground Improvement for constructions in difficult subsoil conditions
 - ❖ Soil Dynamics, Earthquake Geotechnics, Ground Response and Liquefaction Analysis
 - ❖ Soil-Structure Interaction
 - ❖ Rainfall Induced Landslides, Landslide Hazard and Slope Stabilization
 - ❖ Geosynthetic Engineering and Reinforced Soil Structures
 - ❖ Rigid and Flexible Earth Retention Systems
 - ❖ Pavement and Railway Geotechnics
 - ❖ Dam and Embankment Engineering

KNOW THE SUBSURFACE

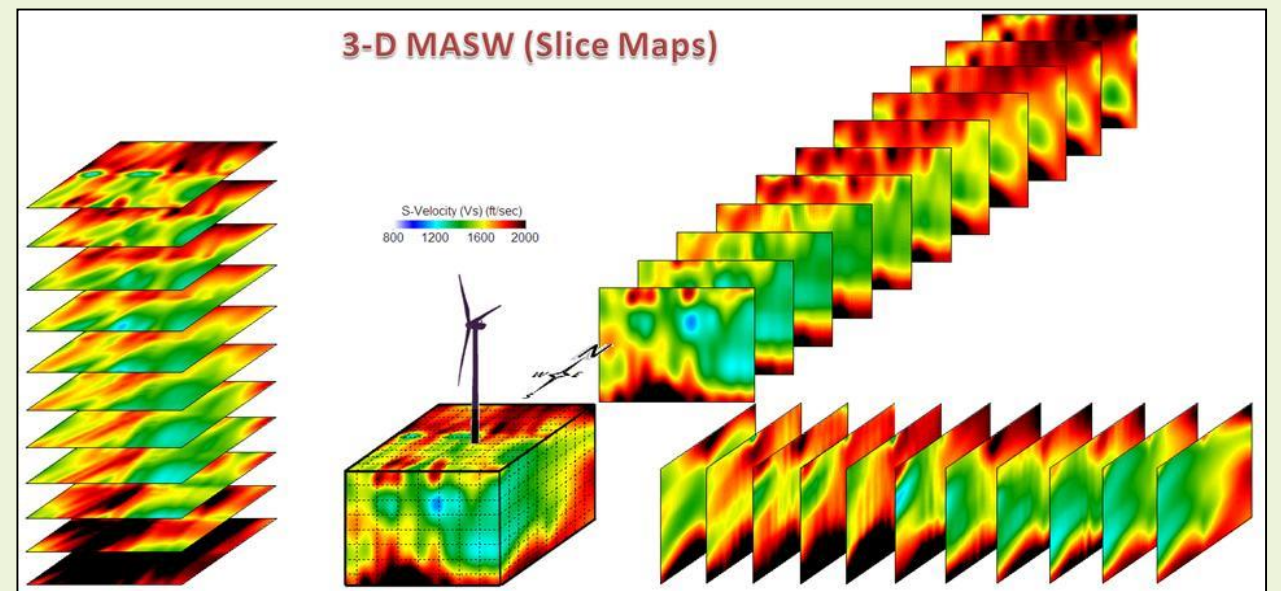
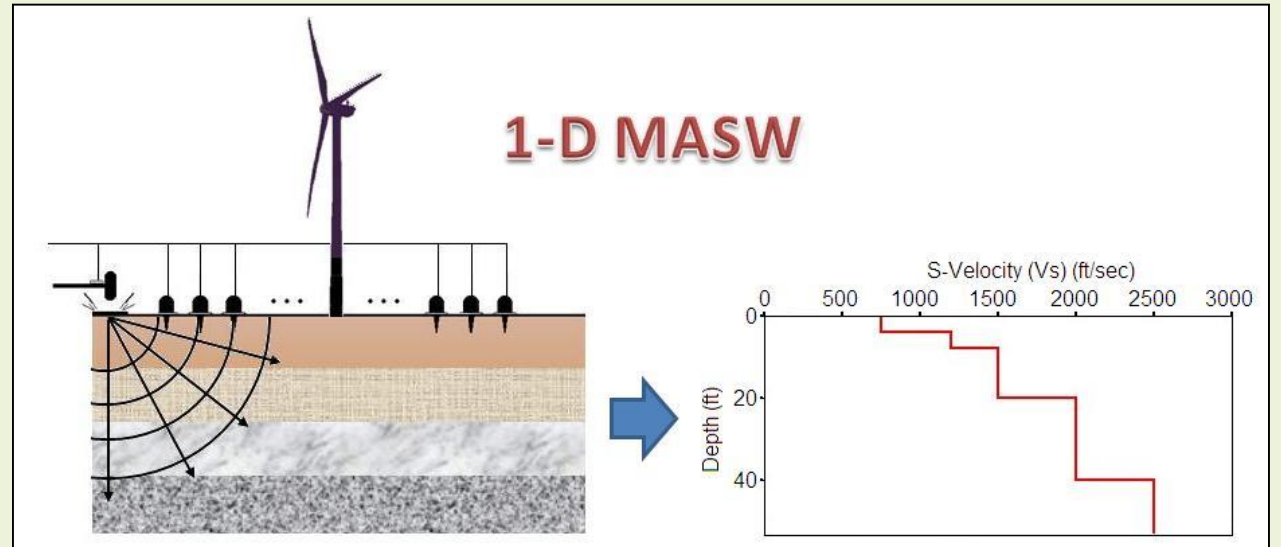
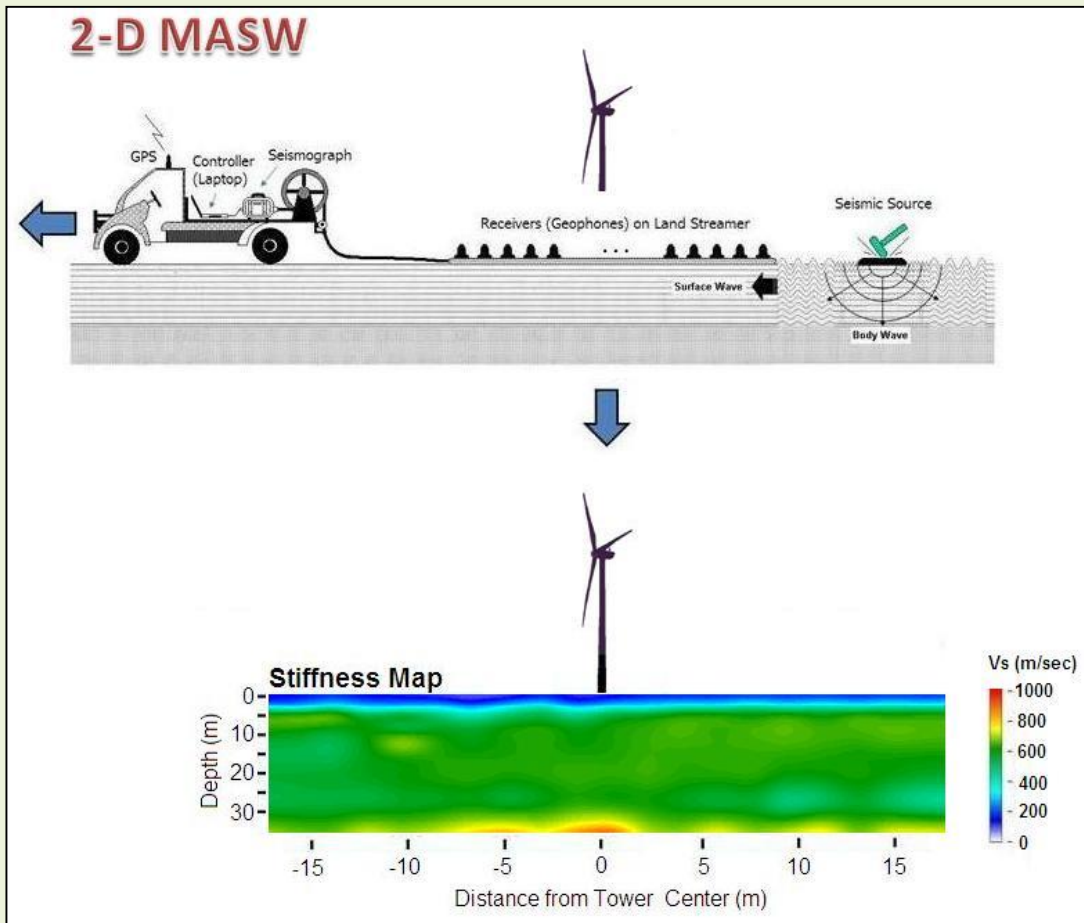
Multichannel Analysis of Surface Waves (MASW) Surveys

- Shear wave velocity profiling of soil substrata
- Operates on the dispersive capacity of soils

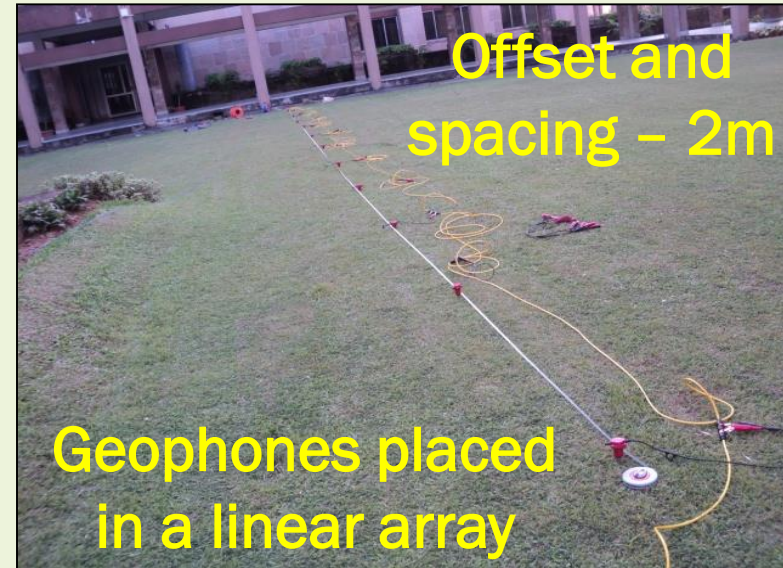
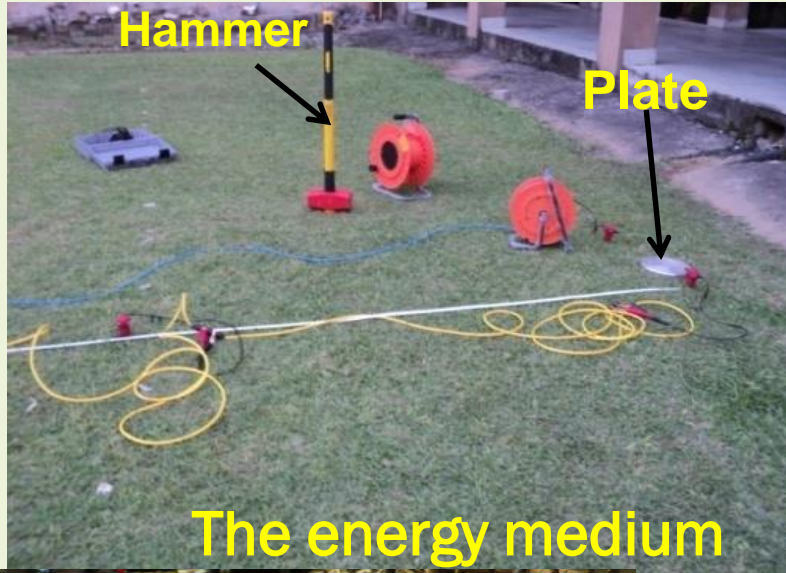


Subsurface Profiling through Inversion Analysis

- ❖ Evaluating ground stiffness
- ❖ 1D, 2D or 3D formats



MASW Survey Components

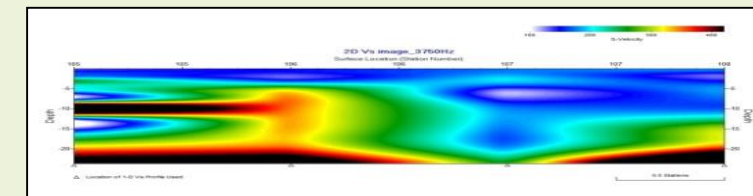
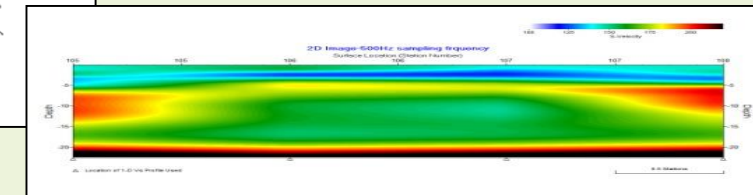
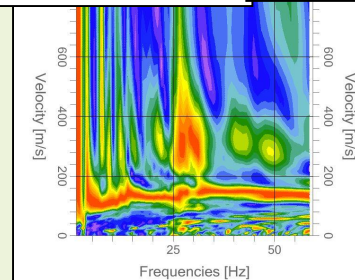
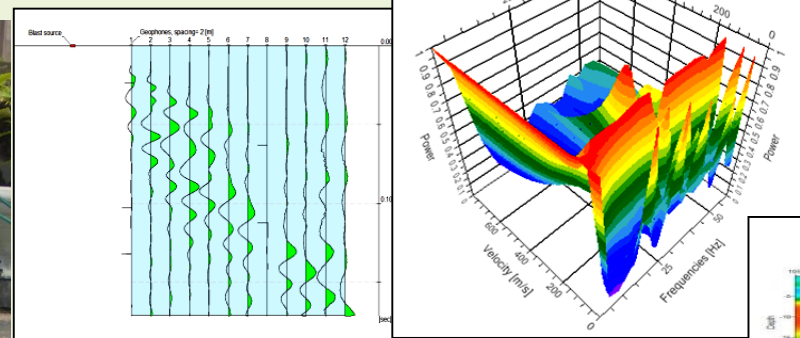
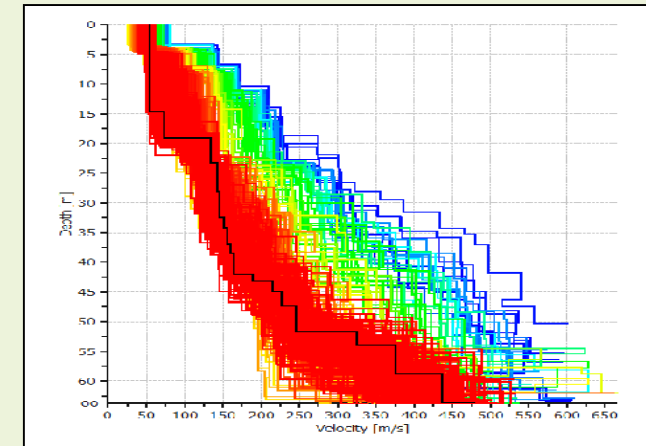


Subsurface Profiling using Active MASW Survey

- MASW (Multichannel Analysis of Spectral Waves)
 - ❖ Seismic exploration technique using NDT
 - ❖ Soil stratigraphy and shear wave velocity profile (1D, 2D or 3D)
- Active MASW Survey
 - ❖ Impact source using sledgehammer or automated drop weight
 - ❖ Depth of investigation: 20-30m with good resolution

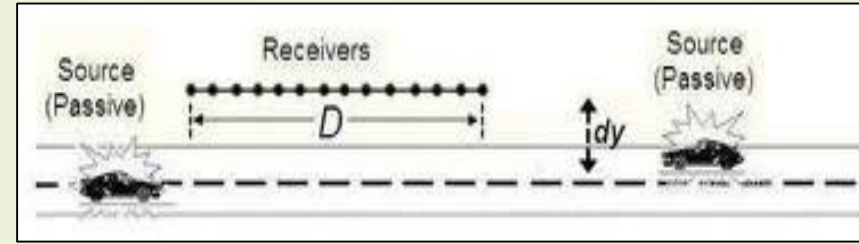


Jumrik

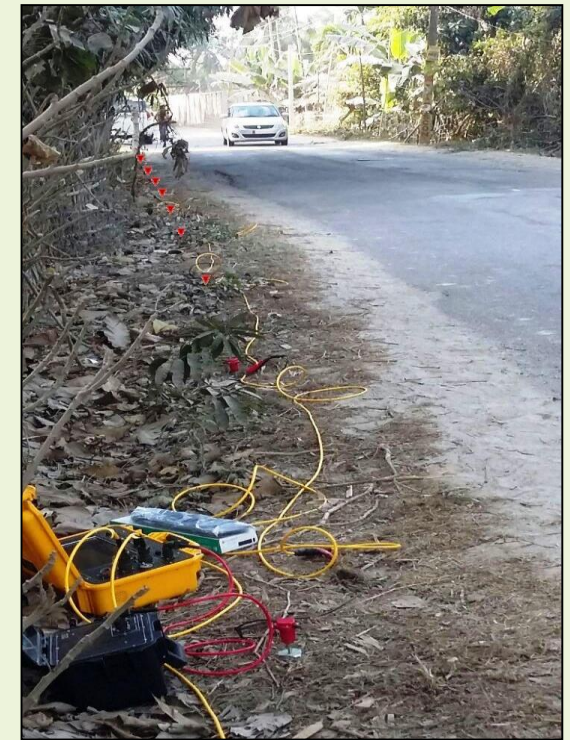
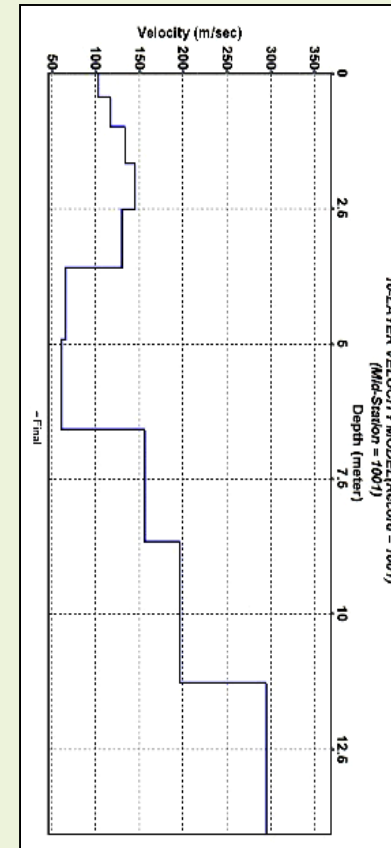
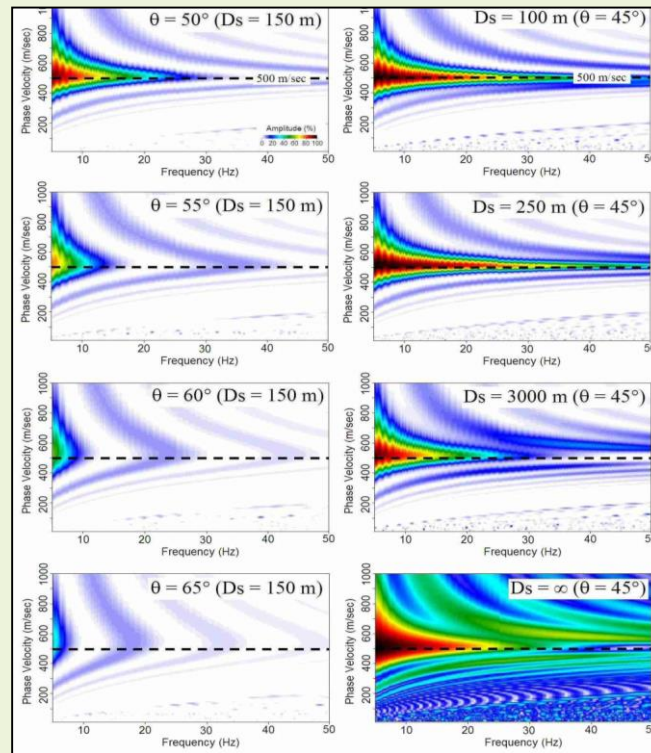
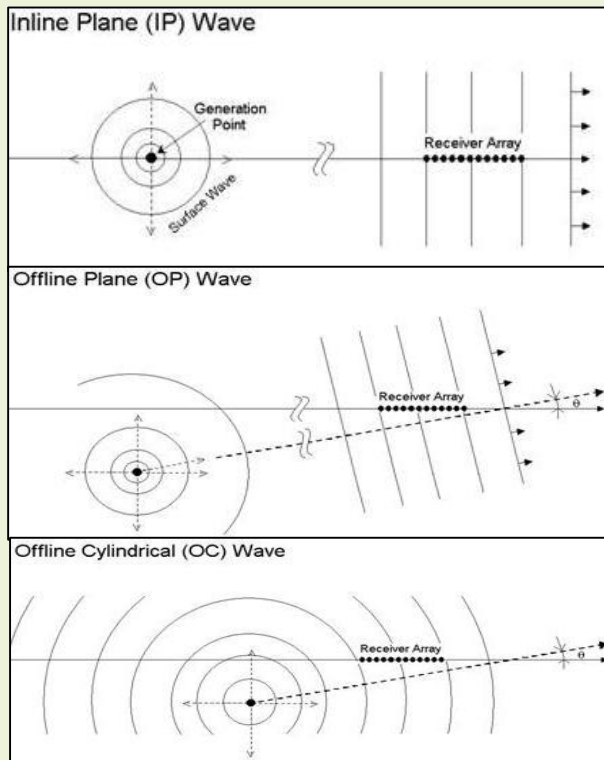


Subsurface Profiling using Passive Roadside MASW Survey

- Passive Roadside MASW Survey
 - ❖ Adopted in populated urban areas
 - ❖ Utilization of vehicle loading as energy source
 - ❖ Depth of investigation: 20-50m (Good resolution)



Dipjyoti

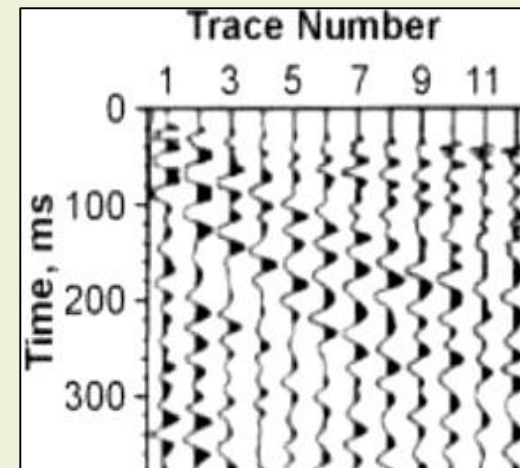
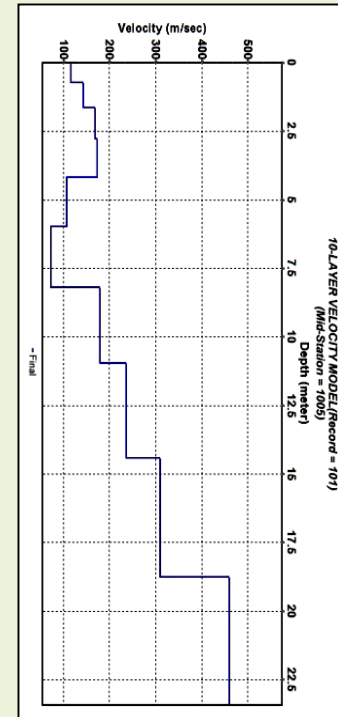
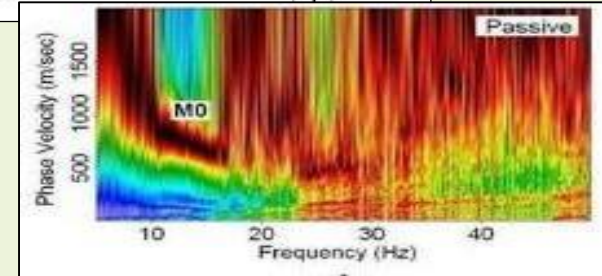
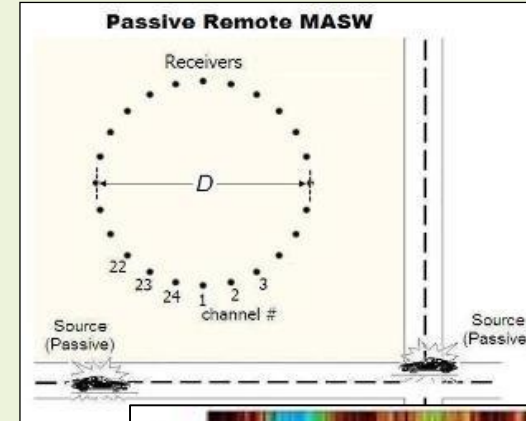


Subsurface Profiling using Passive Remote MASW Survey

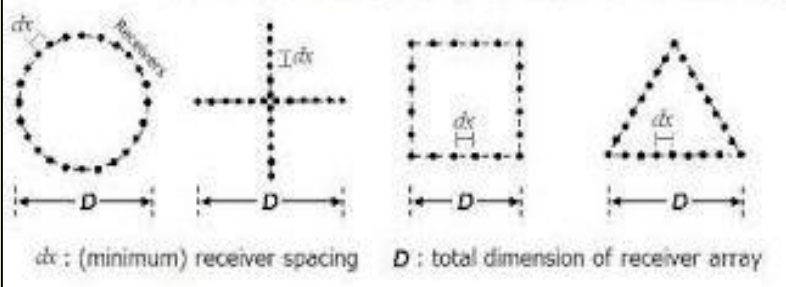
- Passive Remote MASW Survey
 - ❖ 2D Array of geophone receivers
 - ❖ Utilization of long wavelength ambient noise source
 - ❖ Depth of investigation: 30-120m (Good resolution)



Shibayan

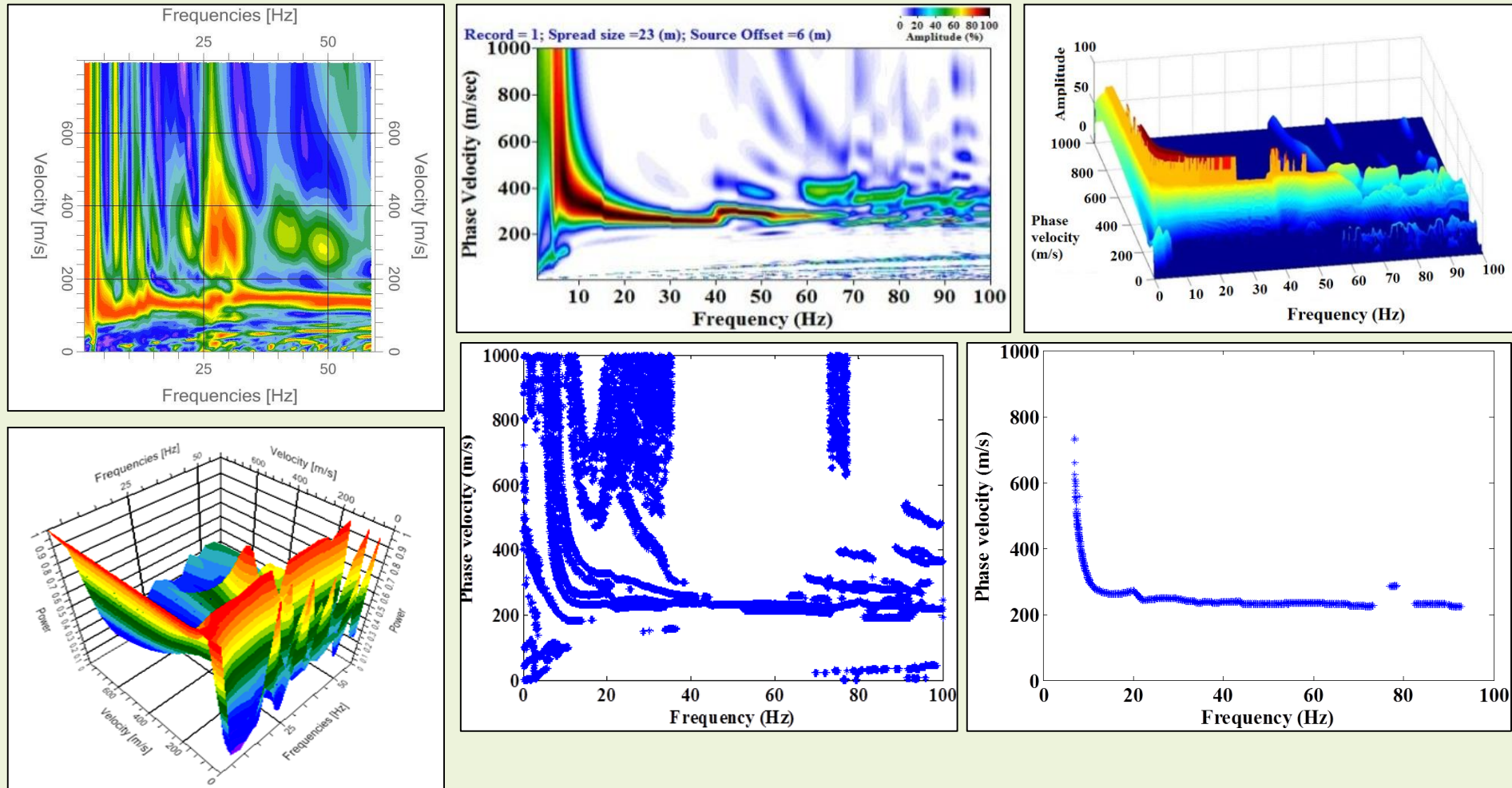


2-D Receiver Arrays for Passive Remote MASW Survey



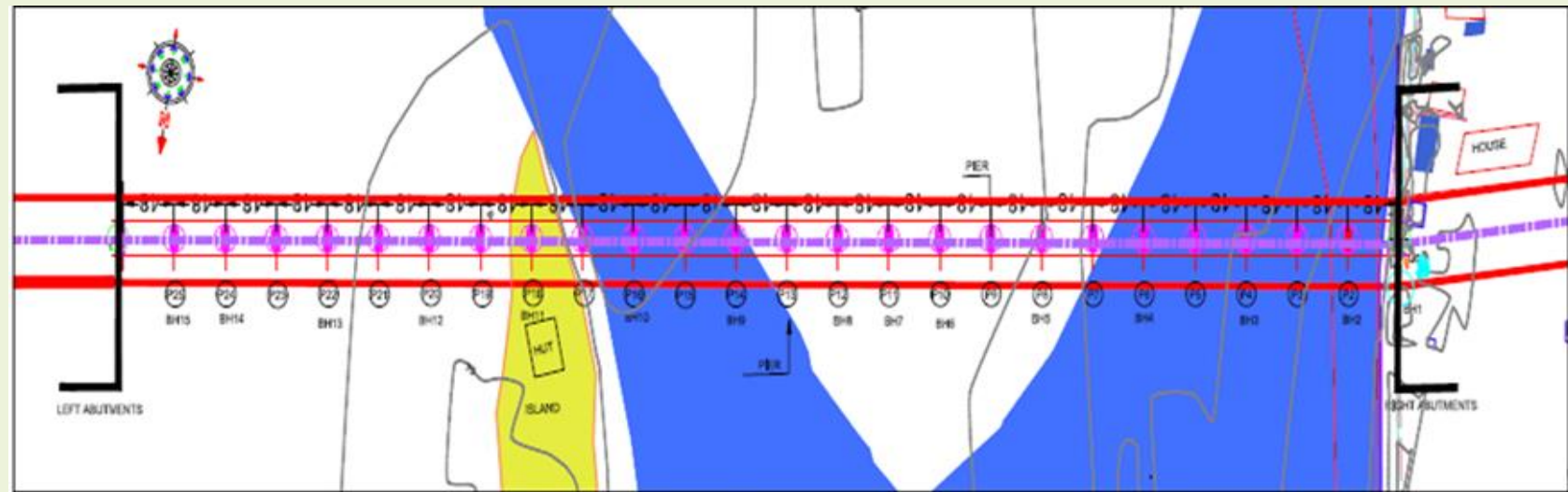
Automated Extraction from 3-D Dispersion Image

- Image processing techniques and Matlab coding



MASW Survey of 1.2 km long Jia-Bharali River Bed

- A new 4-lane (1.2 km) carriageway bridge over Jiya-Bharali River bed
 - ❖ National Highway and Infrastructure Development Corporation Limited (NHIDCL)
 - ❖ Located in Tezpur District, Assam state
 - ❖ Connects NH-52A and NH-37
 - Project completed with Simplex Infrastructures Ltd





Jumrik



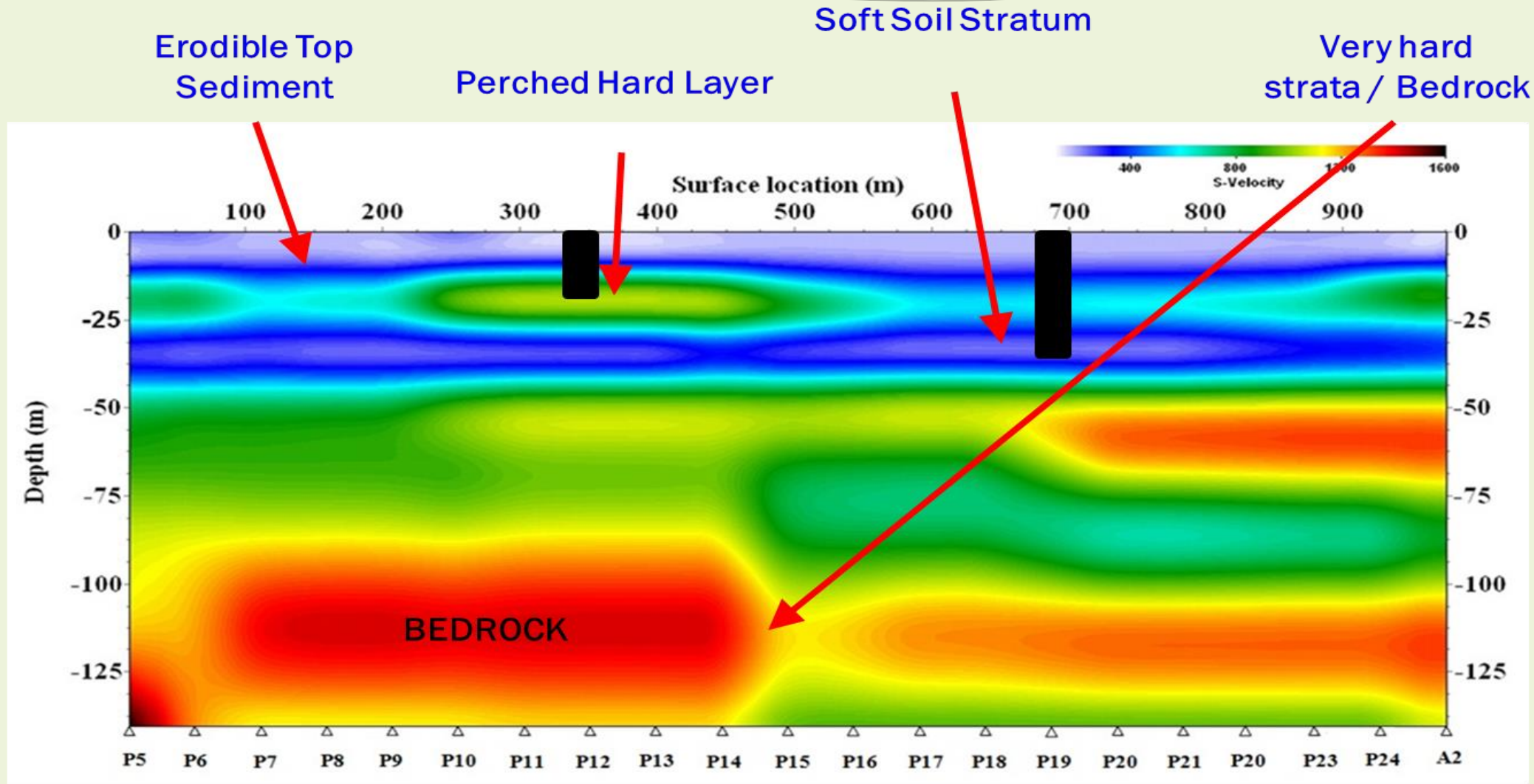
Madhulatha



Rana

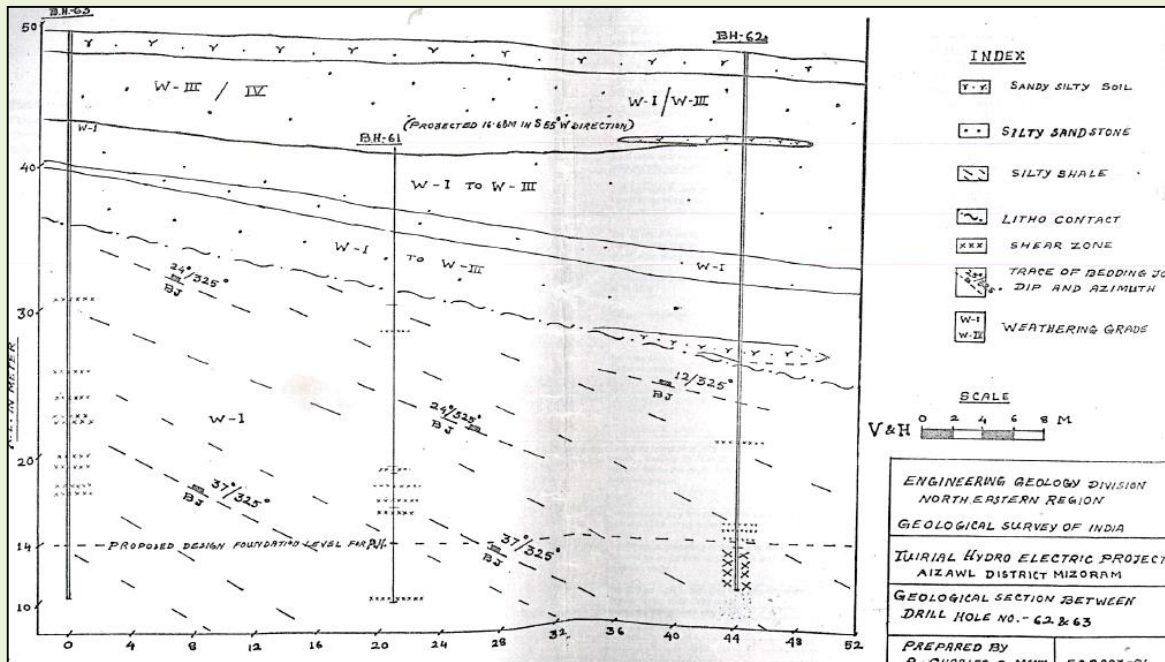


Chiranjib

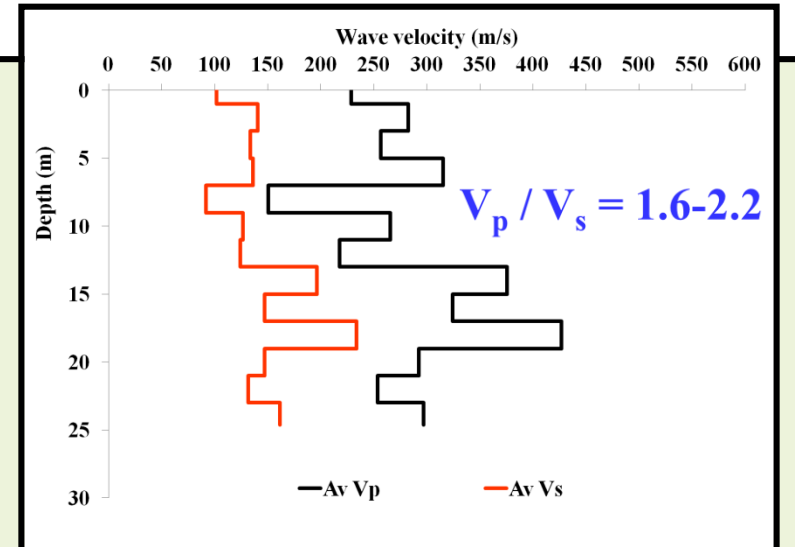
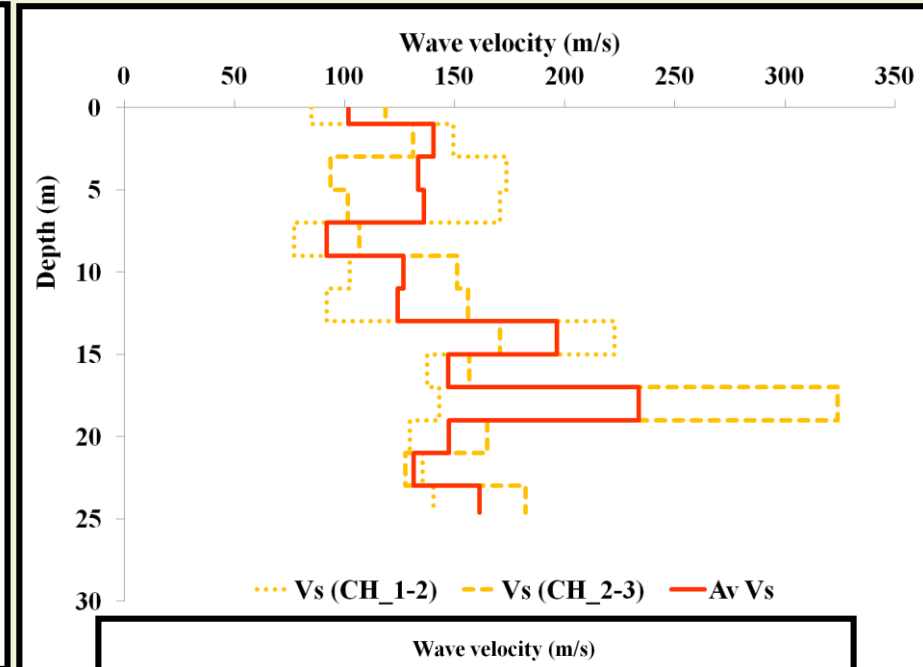
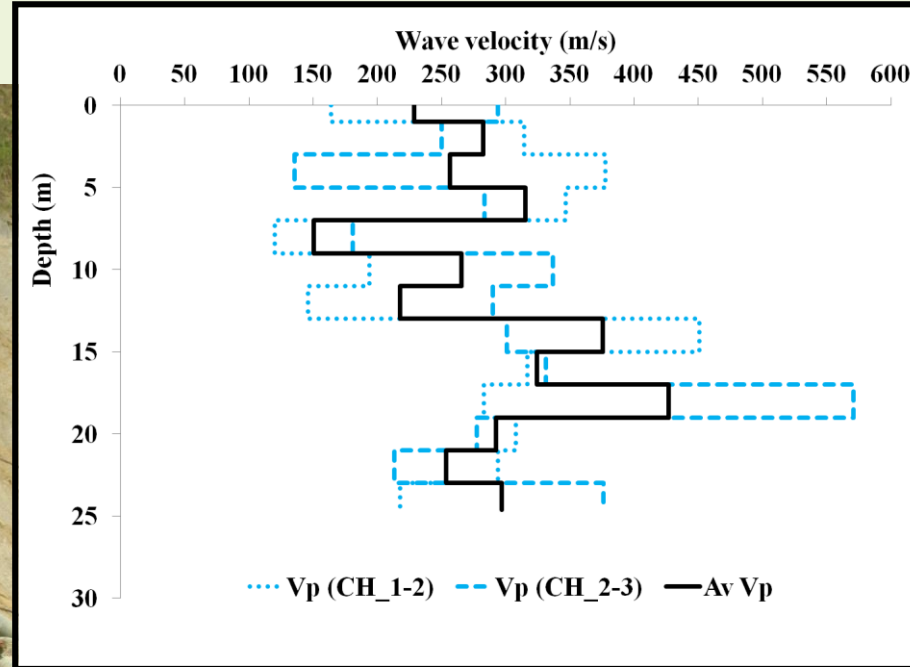
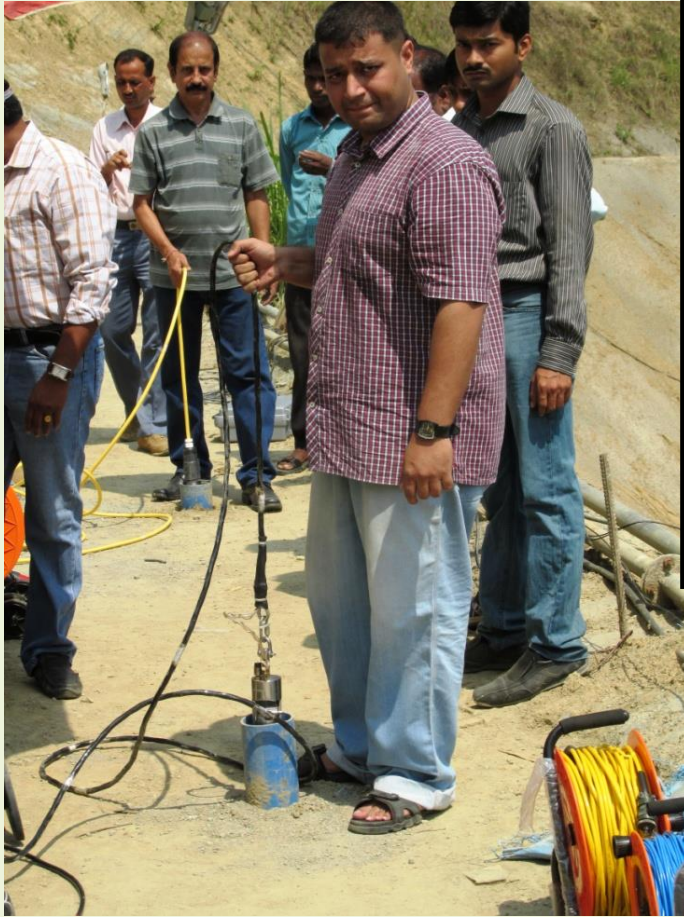


2-D shear wave velocity profile obtained from a roll-along active MASW conducted along the alignment of the proposed bridge over Jia-Bharali (P5-A2)

Tuirial Hydroelectric Power Plant Project, Mizoram



Tuirial Hydroelectric Power Plant Project, Mizoram



Dynamic Response of Typical Soils of NE India

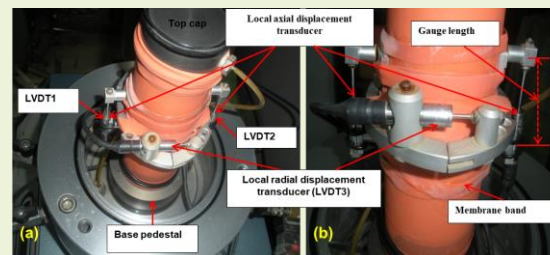
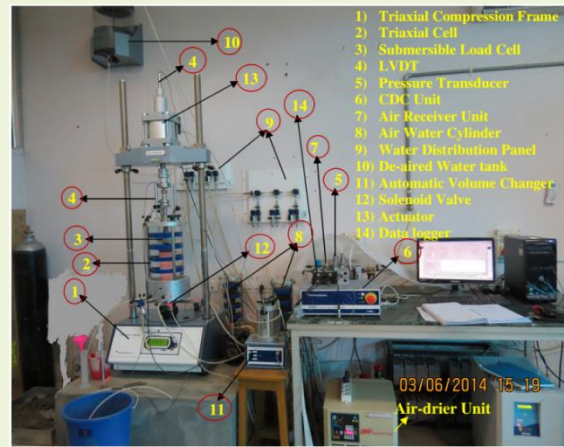
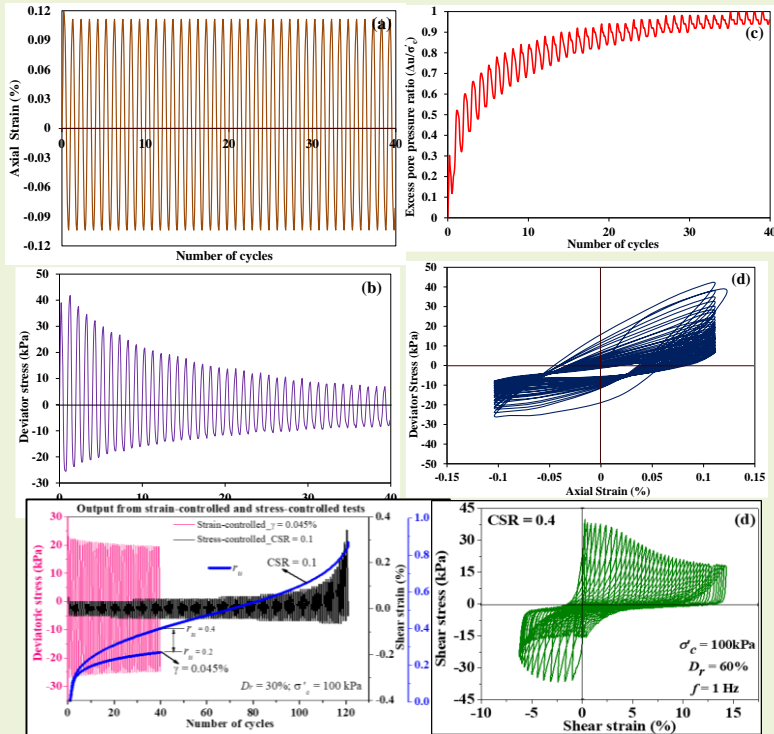
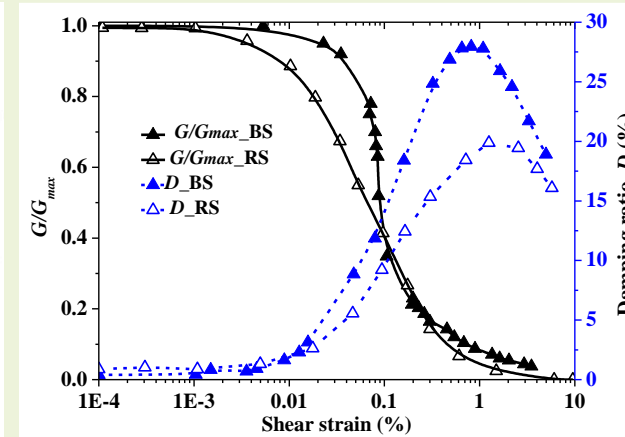
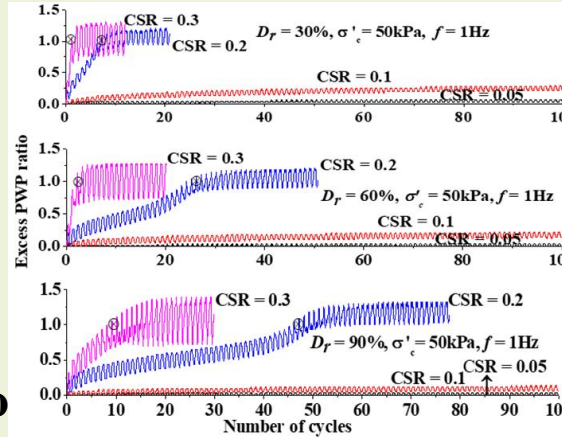


Shiv

• Cyclic Triaxial tests

❖ Strain and Stress controlled tests

- Shear modulus degradation
- Evolution of damping ratio
- Liquefaction potential evaluation



Liquefaction Criteria for Saturated BS

➤ BS ($D_r = 30\%-90\%$) liquefy under the following optimum conditions

❖ $PGA \geq 0.36g$

❖ $CSR \geq 0.3$

❖ $\gamma_{max} > 0.5\%$

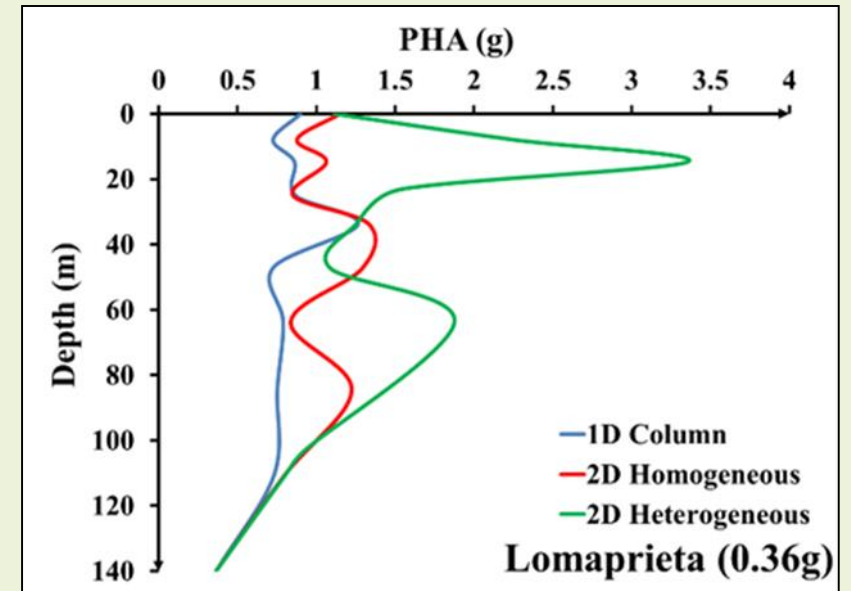
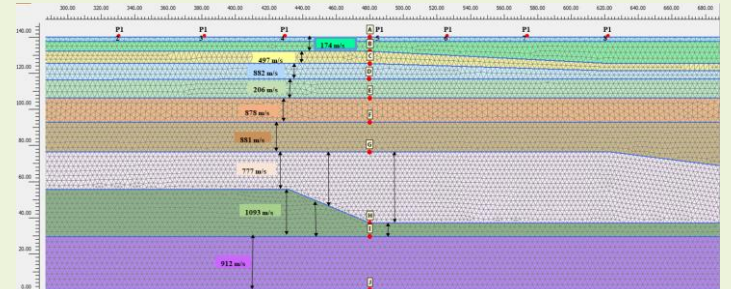
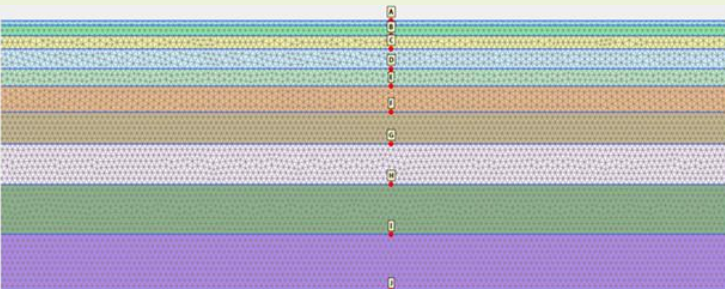
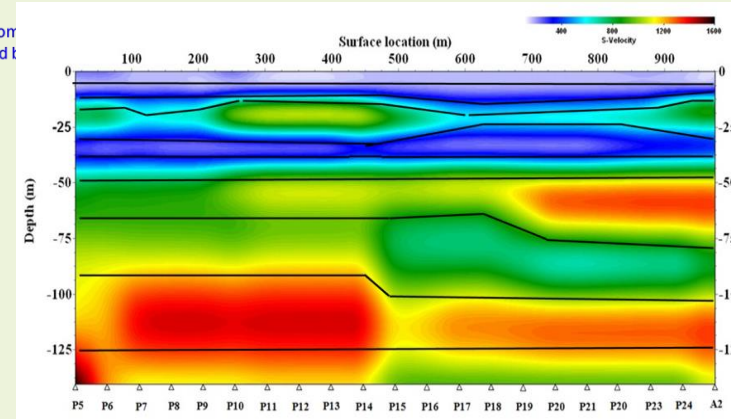
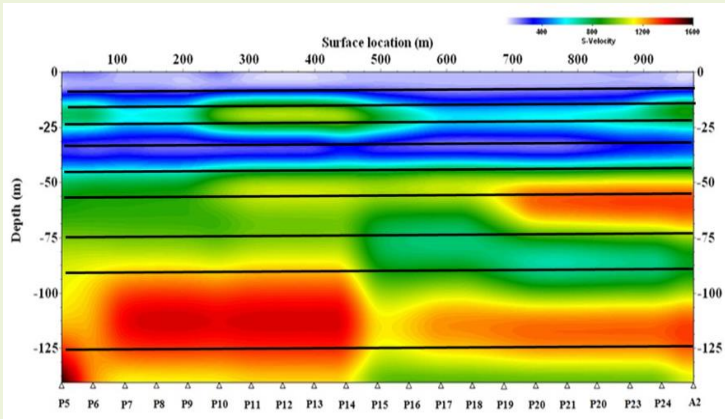
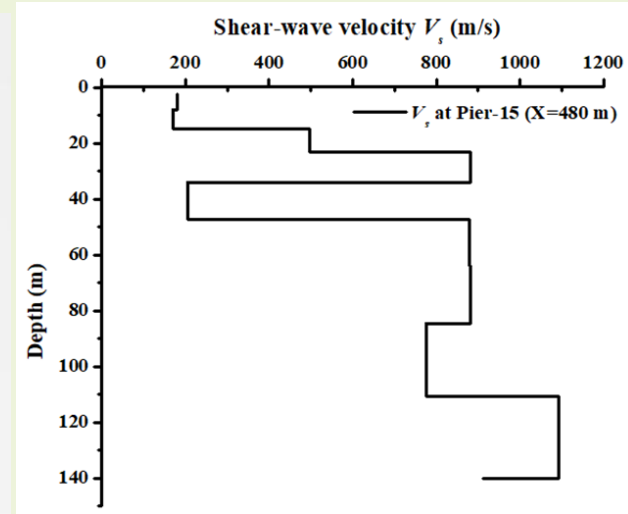
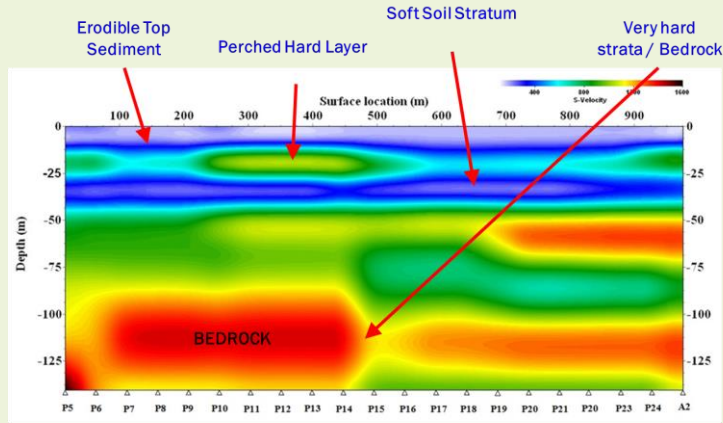
❖ Limiting value of $\gamma = 0.5\%$ is to be adopted for liquefaction evaluation study for BS soil at loose condition

❖ Limiting value of $\gamma = 1.0\%$ is to be adopted for liquefaction study for BS soil at dense condition

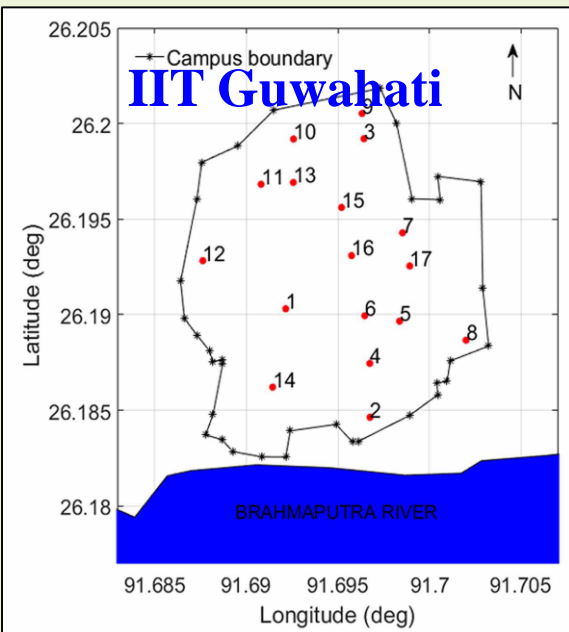
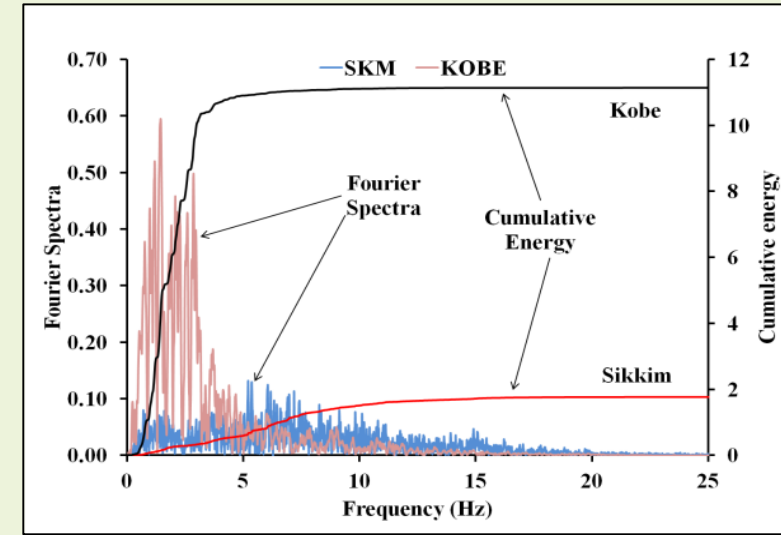
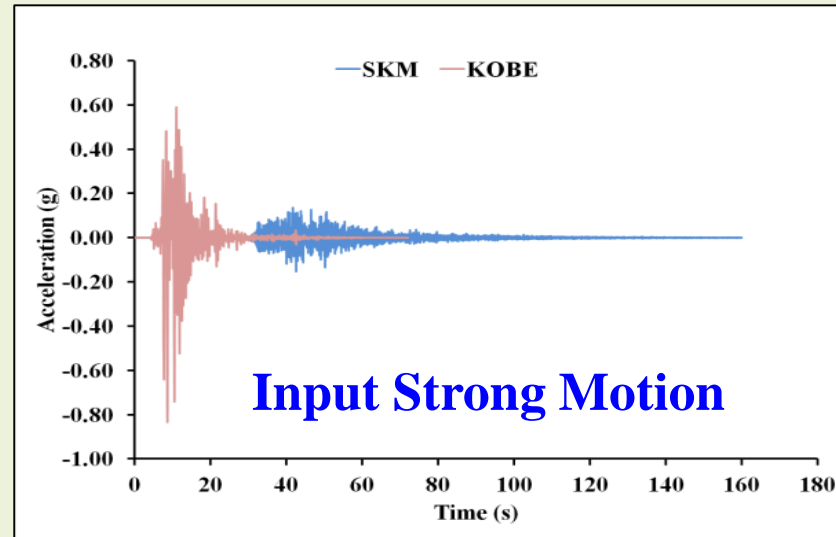
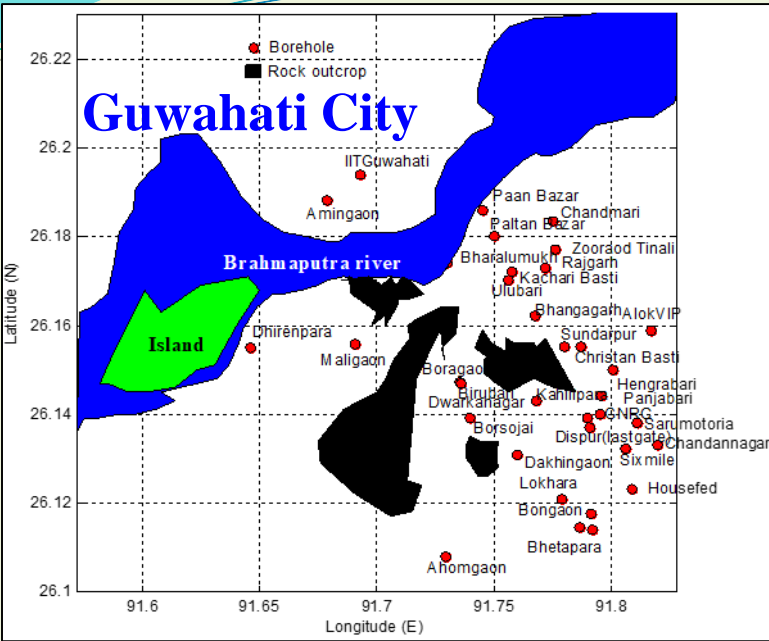


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Representation of Subsurface 1-D and 2-D GRA Models



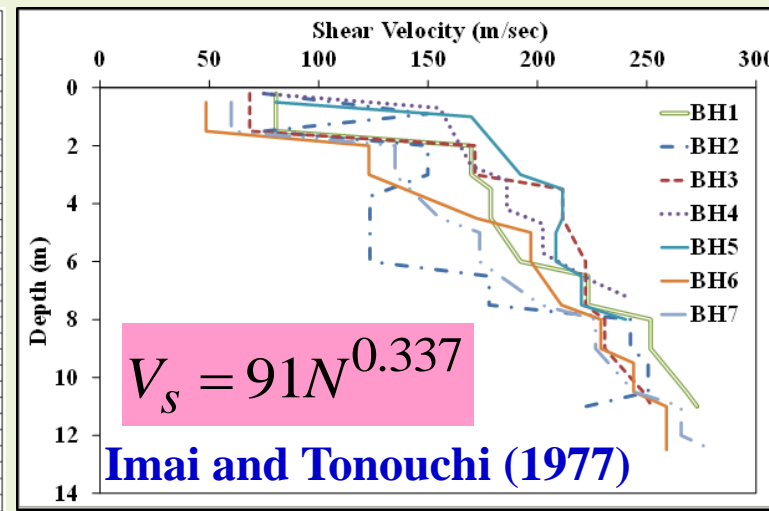
GRA of Guwahati City and IIT Guwahati



Typical Borehole Profile

Layer No.	Thickness (m)	Soil Type	Depth (m)	Field Density (kN/m ³)	Average Field Density (kN/m ³)	SPT N-Value	Shear Wave Velocity, v_s (m/s)	Average v_s (m/s)
1	0.75	Clay	0.75		18.9			78.23
2	0.75		1.5			6	136.5	
3	0.5		2	18.9			147.5	
4	1		3		19.05	4	138.3	151.2
5	1.5		4.5			7	164	
6	0.5		5	19.2			138	
7	1		6		18.4	2	112	112
8	1.5		7.5			2	112	
9	0.5		8	17.6			138	
10	1		9			7	164	185.3
11	1.5		10.5		18.55	15	206.7	
12	0.5		11	19.5			192	
13	1		12			9	177	174
14	1.5		13.5		19.25	8	170.75	
15	0.5		14	19			209	
16	1		15			27	247.1	248.5
17	1.5		16.5		19.35	28	250	
18	0.5	17				245.7		
19	0.5	17.5	19.7					
20	0.5	18			25	241.5	244.3	
21	1.5	19.5		19.65	27	247.1		
22	0.5	20	19.6			262.8		
23	1	21			40	278.5	282.6	
24	1.5	22.5		20.05	44	286.7		
25	0.5	23	20.5			296.7		
26	1	24		21.05	55	306.8	315.5	
27	1.5	25.5			66	324.3		
28	0.5	26	21.6			324.3		

V_s Profile



Fourier and Energy Spectra

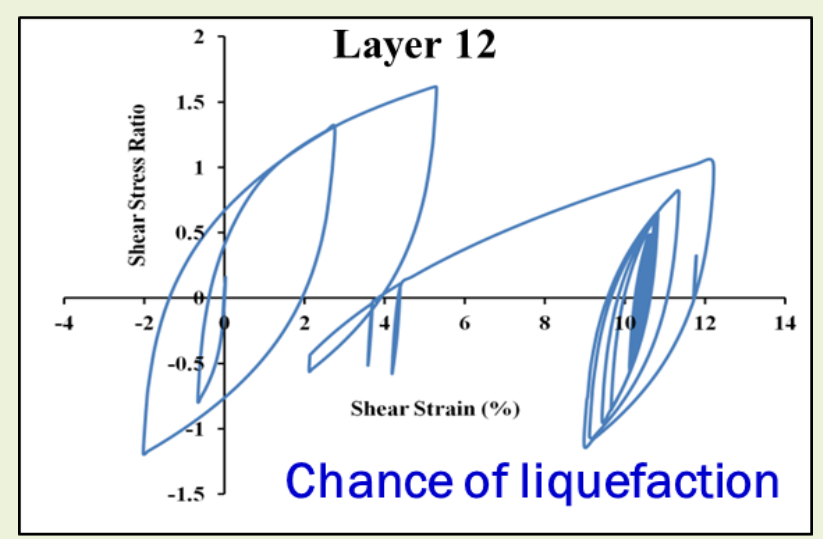
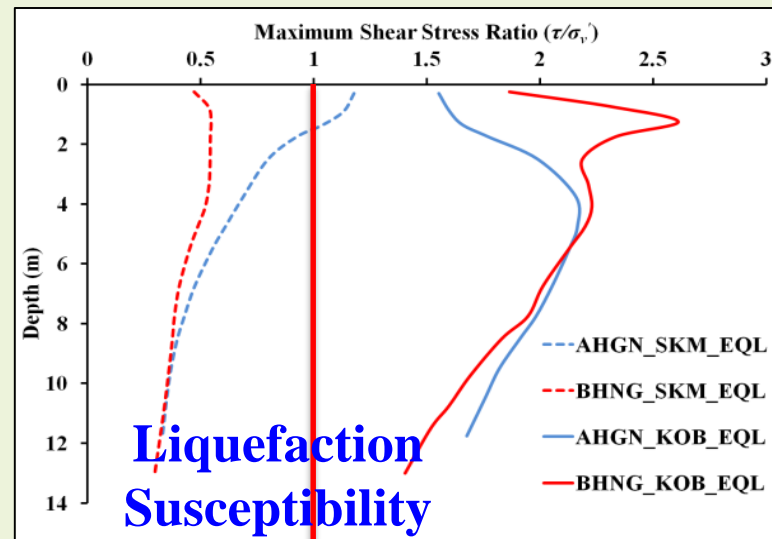
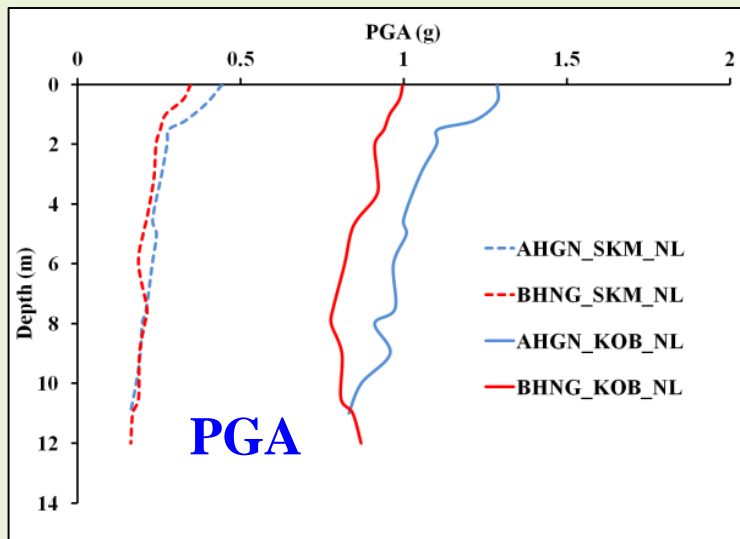
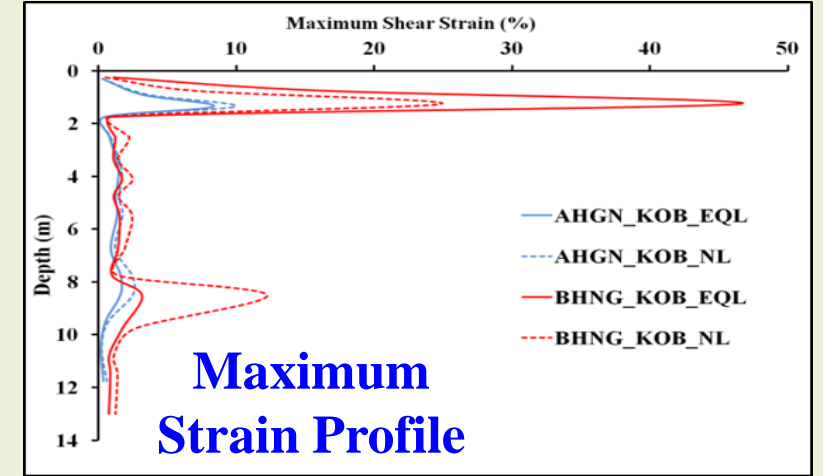
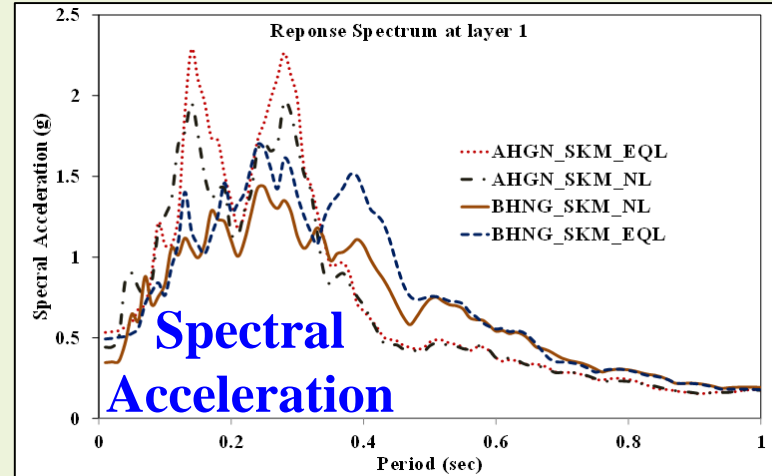
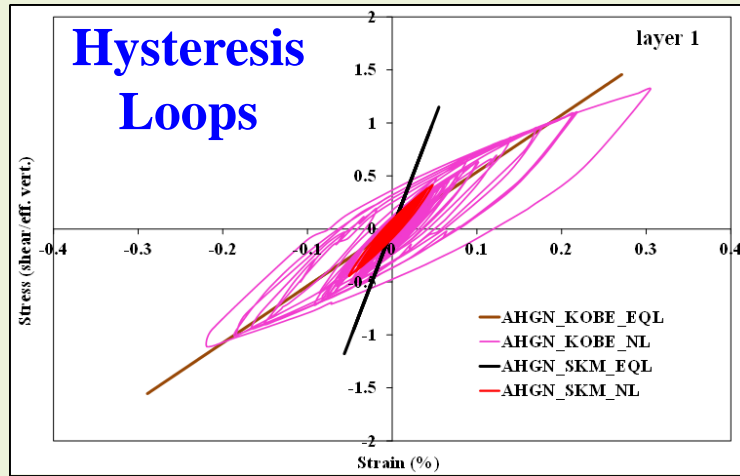


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Imai and Tonouchi (1977)

Typical GRA Results and Representations

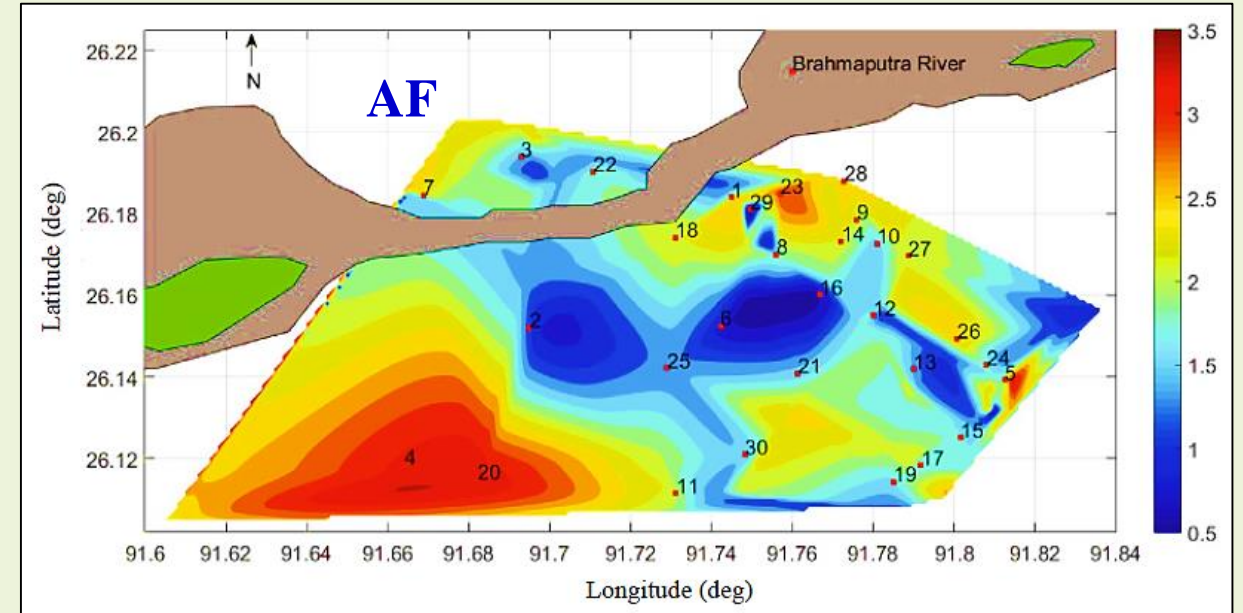
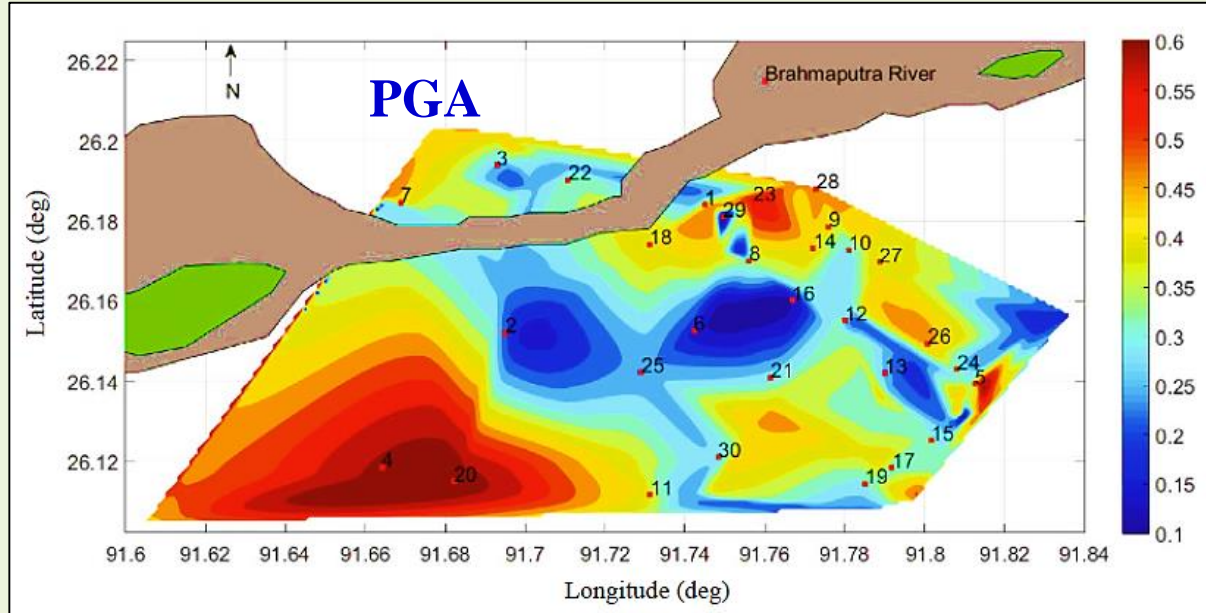


Typical PGA and AF Contour Map of Guwahati City

- Manifestation of Local Site Effects due to varying geology
 - ❖ Local Amplification and Attenuation of bedrock motion



Shiv



Nepal Eq: PBRA = 0.18g

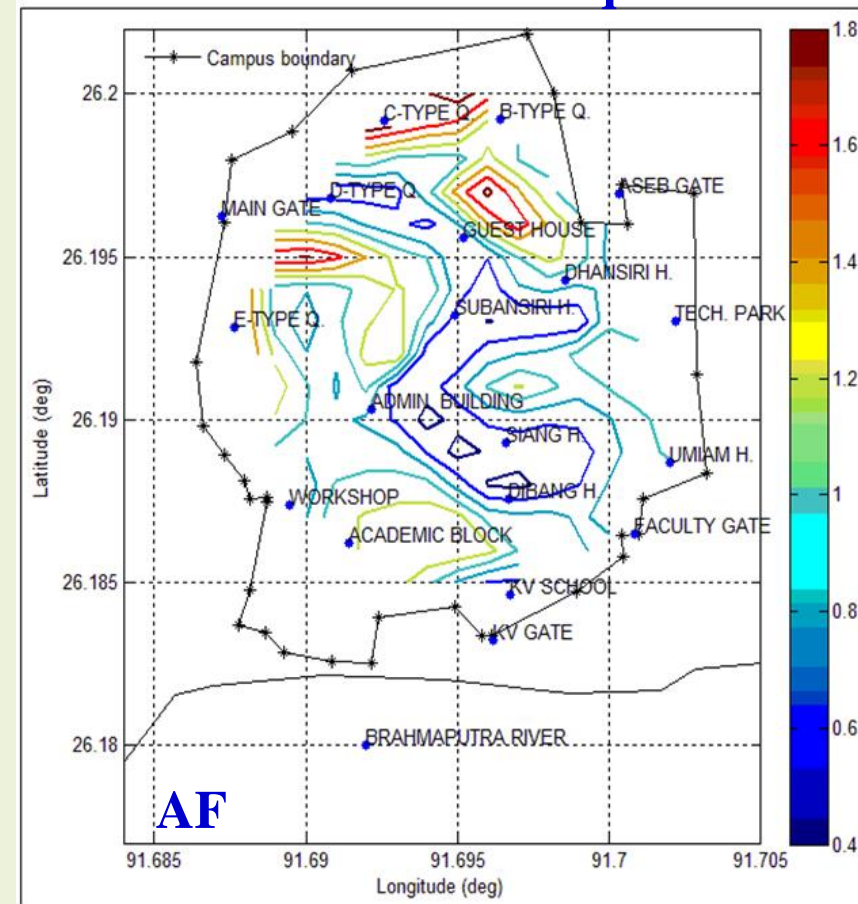
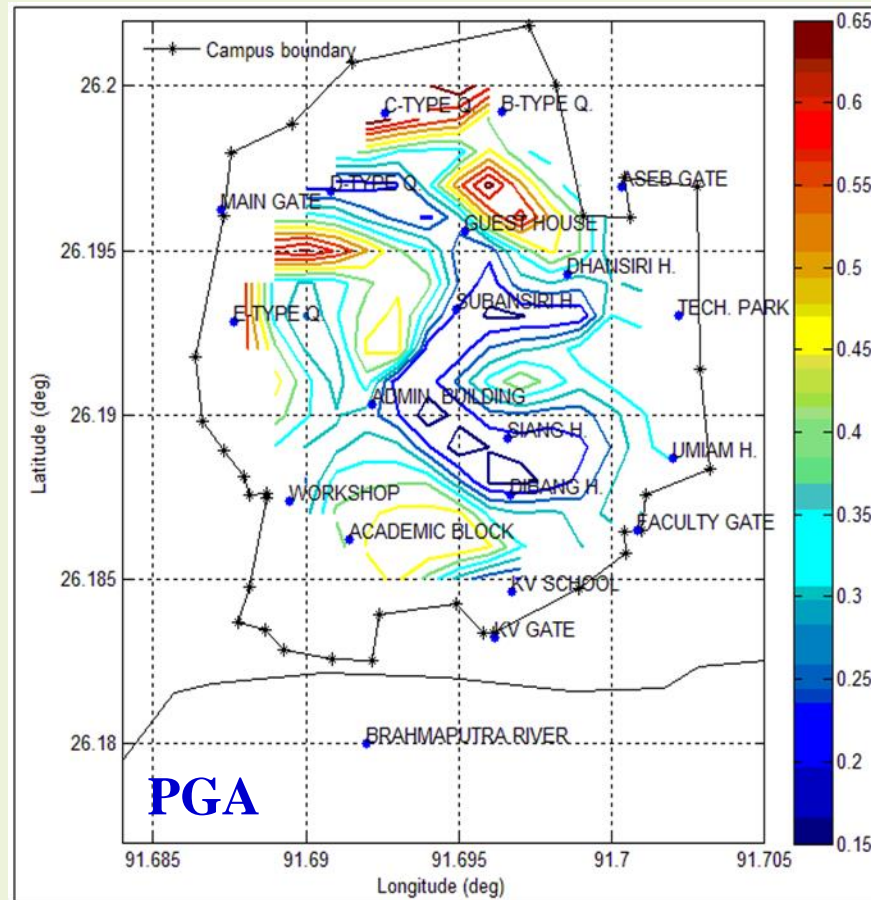
Typical PGA and AF Contour Maps of IIT Guwahati

- Manifestation of Local Site Effects due to varying geology
 - ❖ Local Amplification and Attenuation of bedrock motion



Devdeep

Sikkim Eq: PBRA = 0.36g



Liquefaction FoS Maps of IIT Guwahati

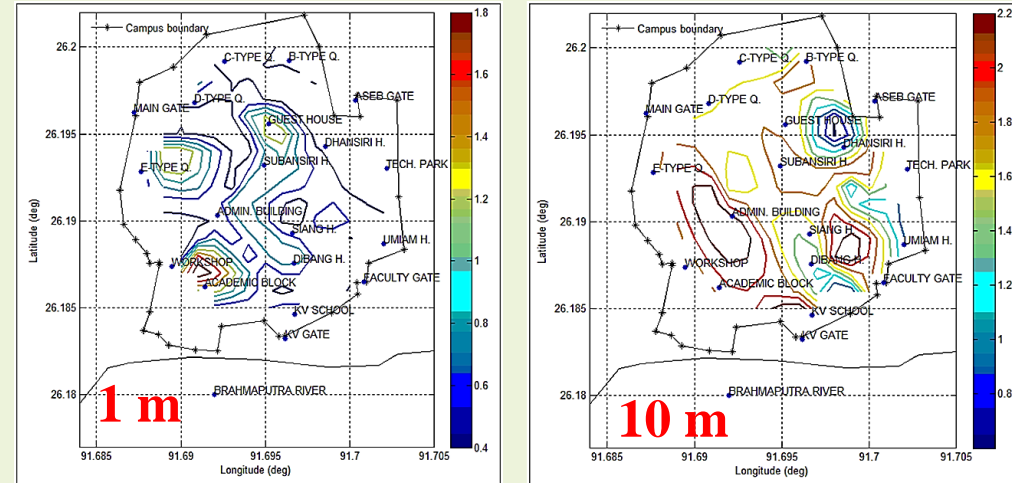
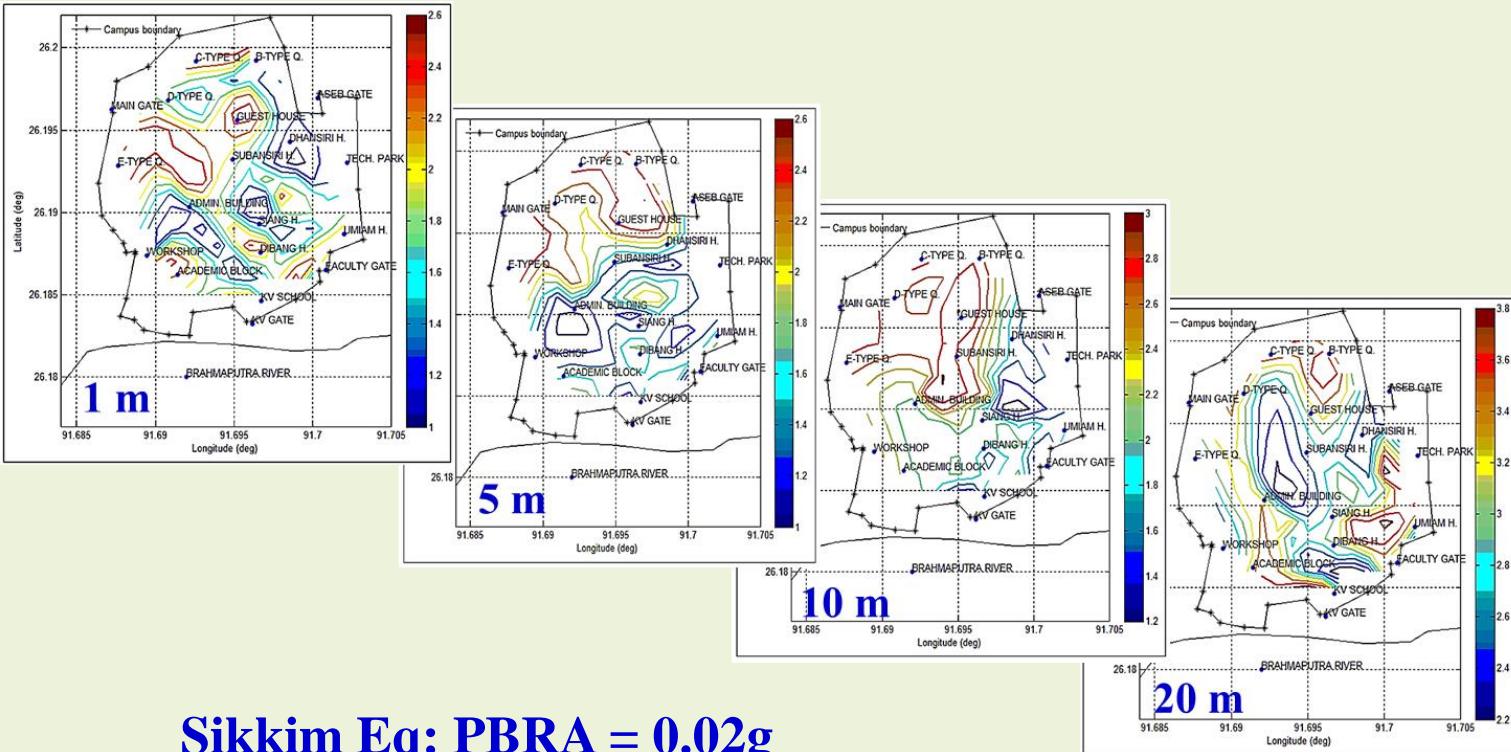
- Influence of peak bedrock acceleration

- ❖ No liquefaction for lower bedrock motions

- FoS > 1 at all places in the campus

- ❖ Substantial liquefaction at higher bedrock motions

- FoS < 1 at many places in the campus at both shallow and deeper strata

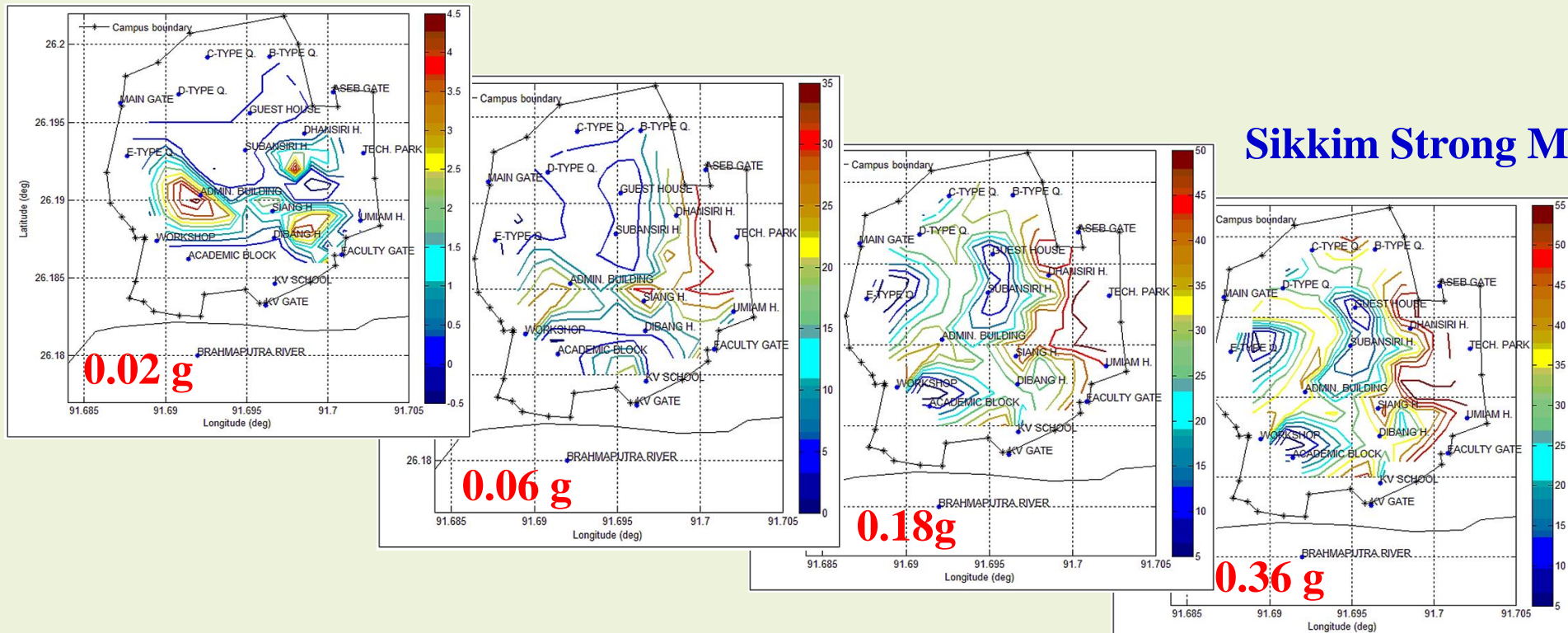


Sikkim Eq: PBRA = 0.36g

Sikkim Eq: PBRA = 0.02g

Liquefaction Potential Index (LPI) of IIT Guwahati

- Indicates susceptibility to liquefaction
 - $LPI < 5 \rightarrow$ No liquefaction
 - $5 < LPI < 15 \rightarrow$ Moderate liquefaction
 - $LPI > 15 \rightarrow$ Severe liquefaction

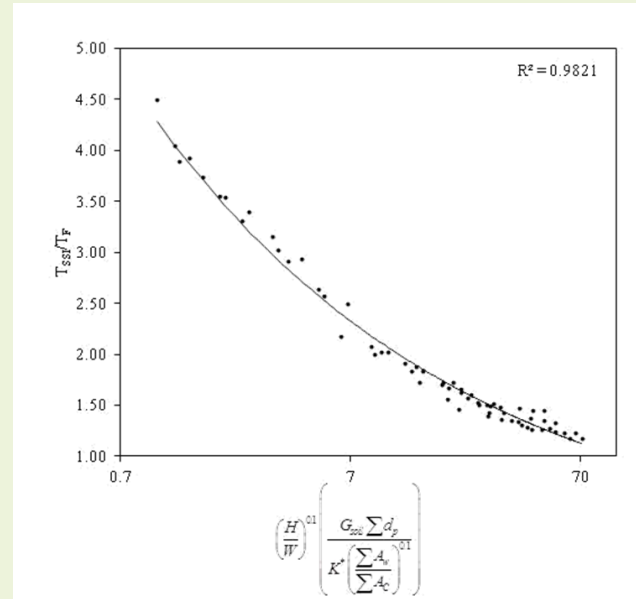
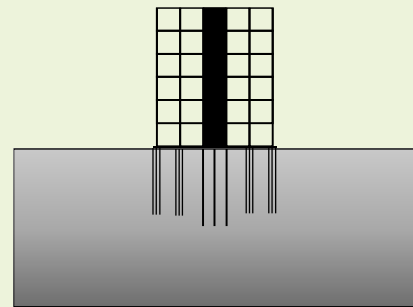


Seismic Soil-Structure Interaction

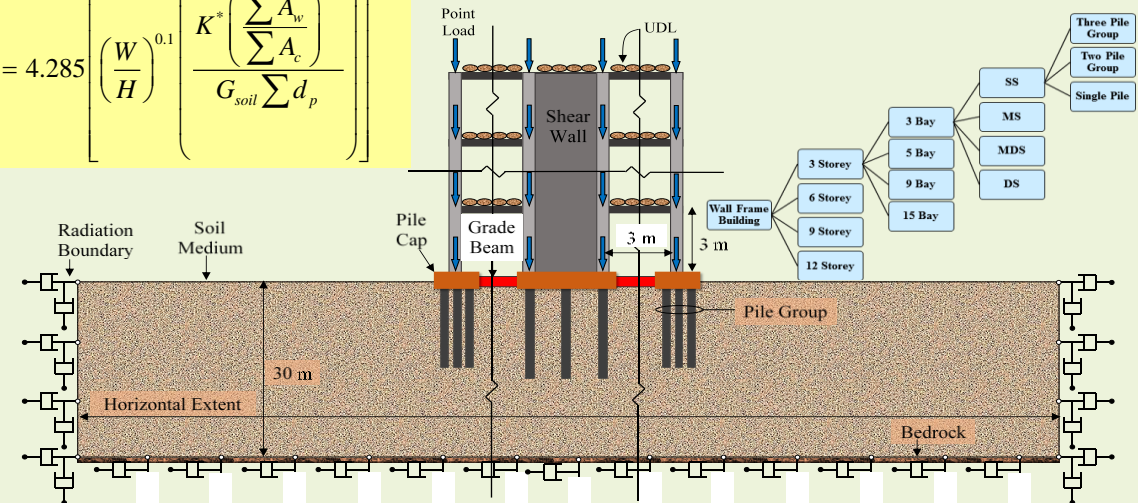
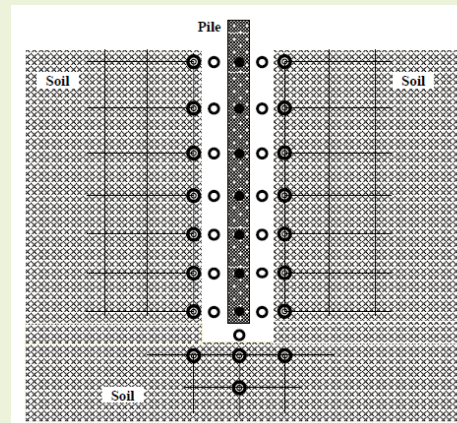
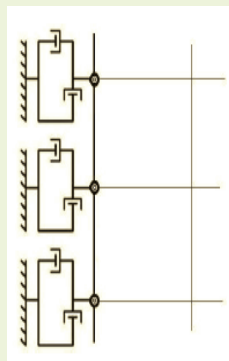
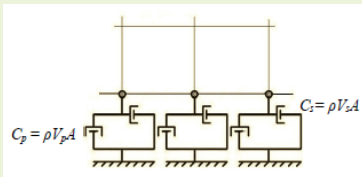
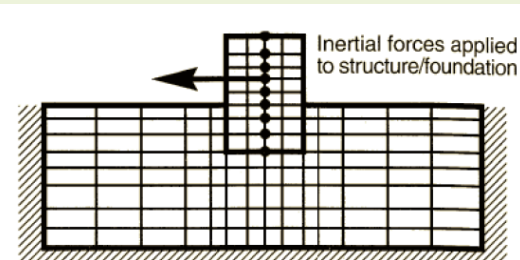
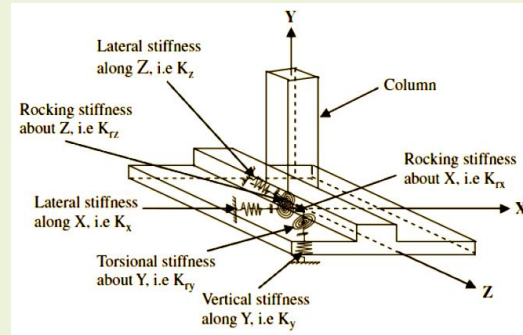
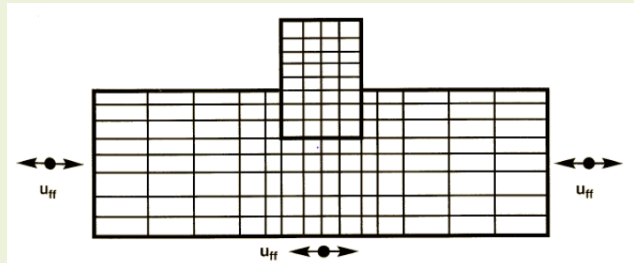
- Soil-Structure Interaction
 - ❖ Seismic analysis of integral abutment RC bridges
 - ❖ Seismic behavior of RC wall-framed buildings
 - ❖ Kinematic and Inertial interaction of building foundations



Nishant



$$\frac{T_{SSI}}{T_F} = 4.285 \left(\frac{W}{H} \right)^{0.1} \left[\frac{K^* \left(\frac{\sum A_w}{\sum A_c} \right)^{0.1}}{G_{soil} \sum d_p} \right]^{0.314}$$

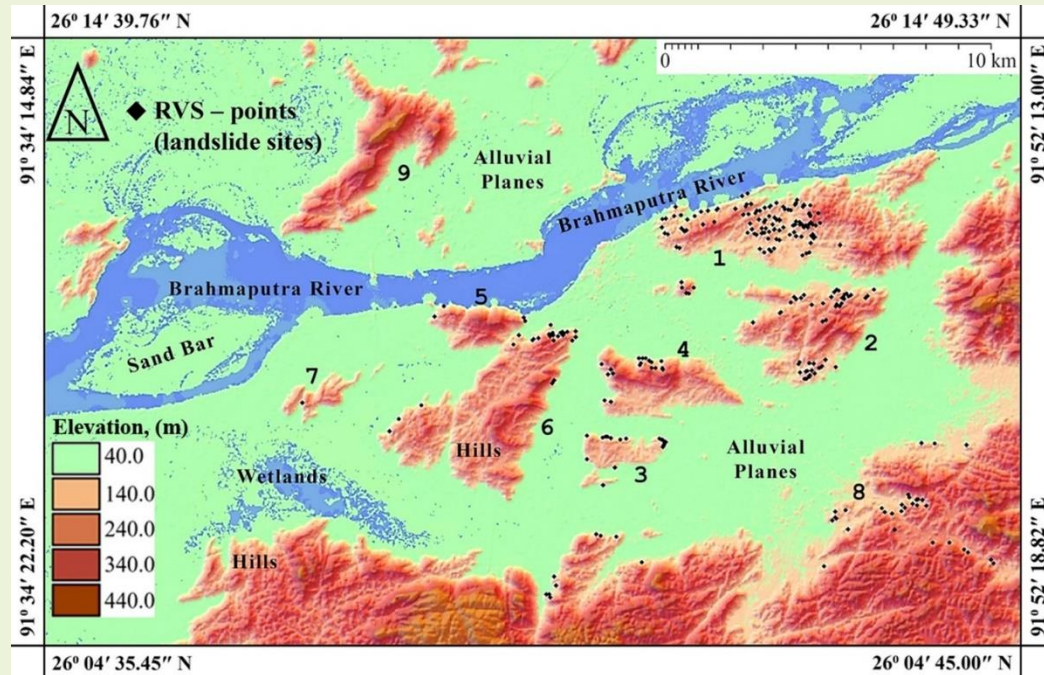


Rainfall Induced Landslide Hazard of Guwahati City

- Study area and characterization of hillslope soils

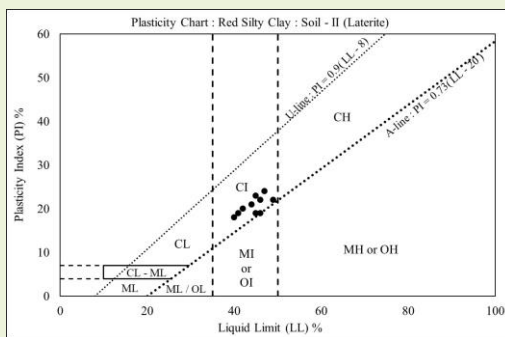
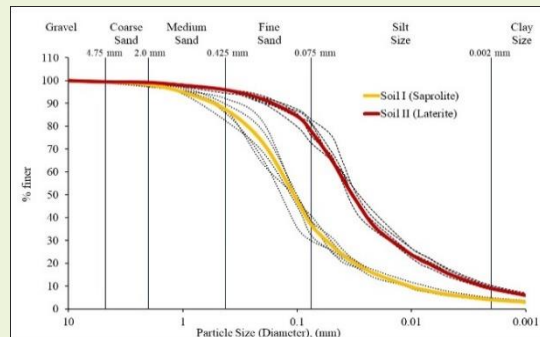
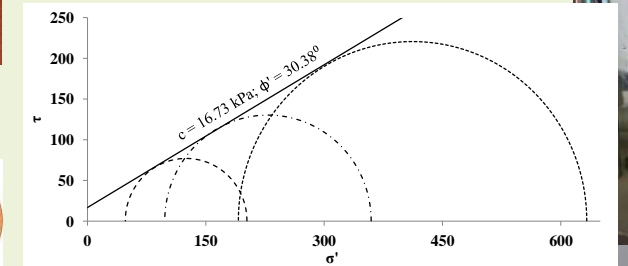
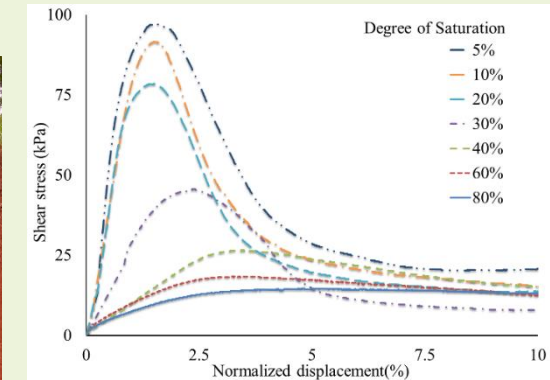
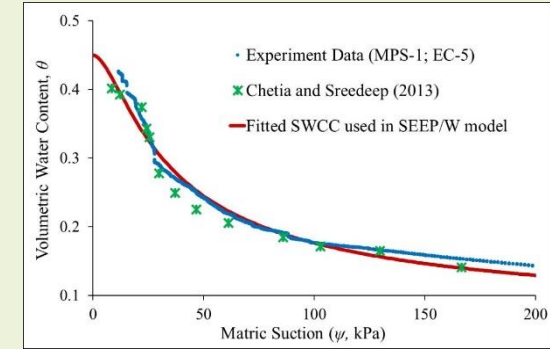


Chiranjib



8 major hill series:

- (1) Nabagraha and Sunsali hill series
- (2) Japorigog hill
- (3) Sonaighuli and Jutikuchi hill series
- (4) Narakashur hill
- (5) Nilachal hill
- (6) Fatasil hill
- (7) Jalukbari hill
- (8) Khanapara hill
- (9) Agyathuri hills



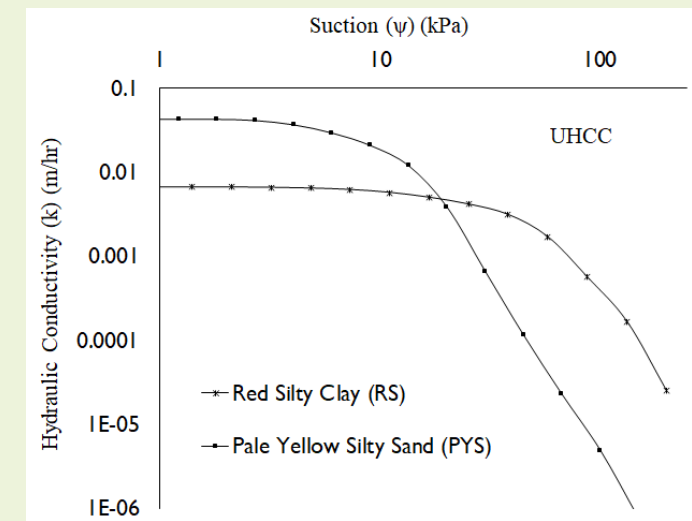
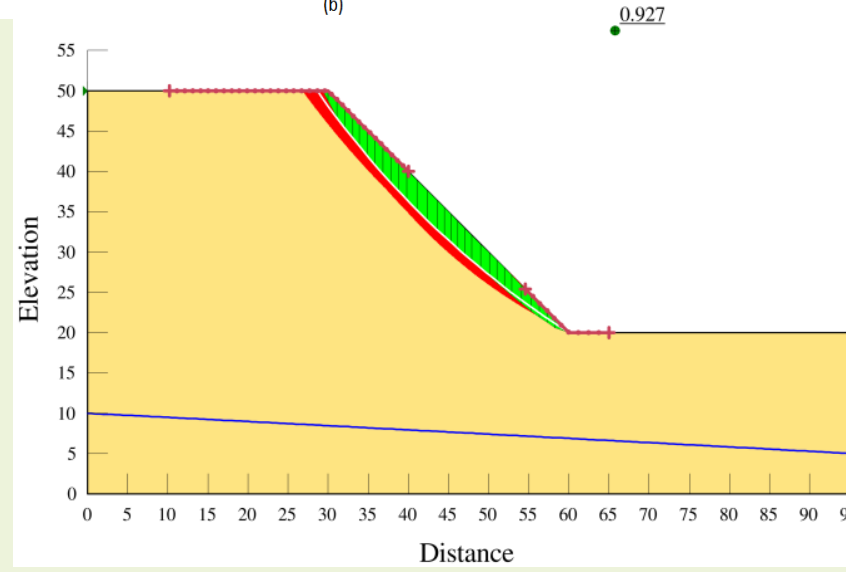
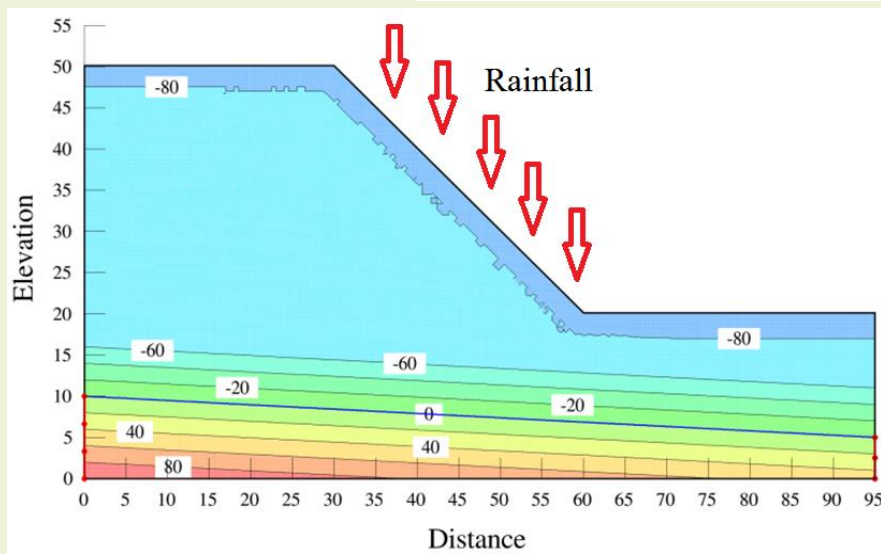
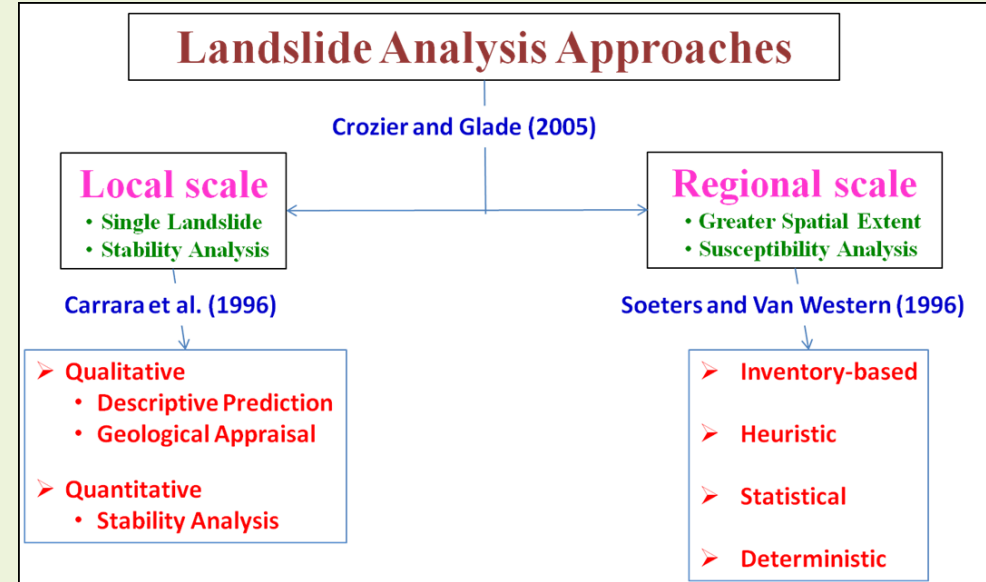
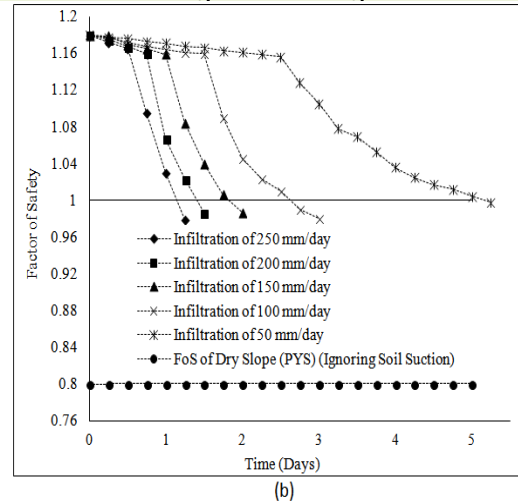
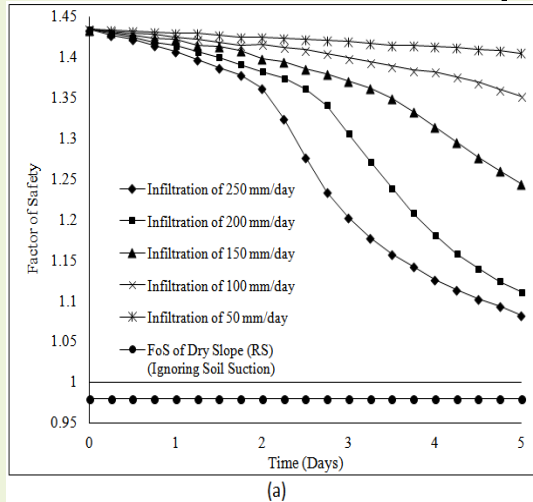
Rainfall Induced Landslide Hazard of Guwahati City

Local-scale deterministic slope stability analysis

Applied Infiltration

Five different rates of infiltration

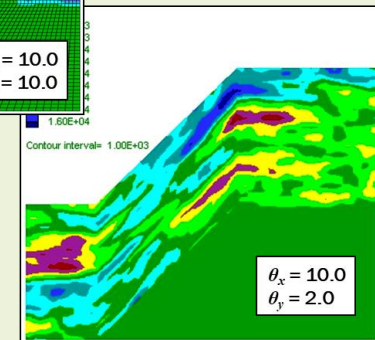
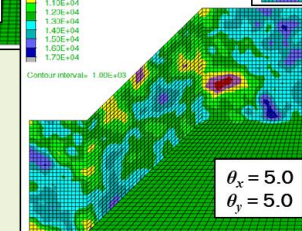
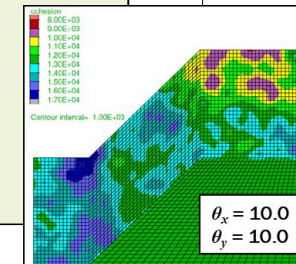
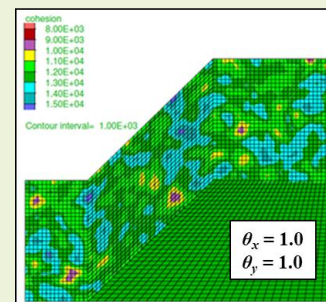
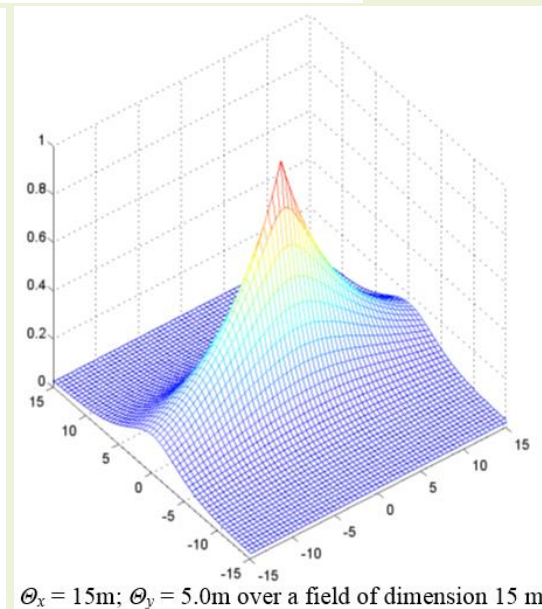
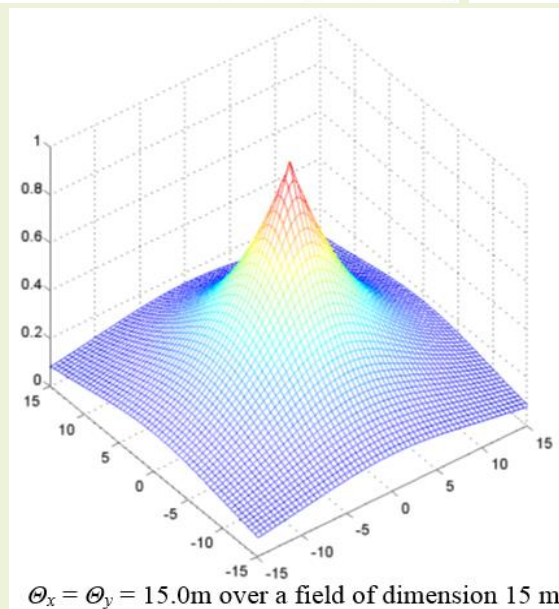
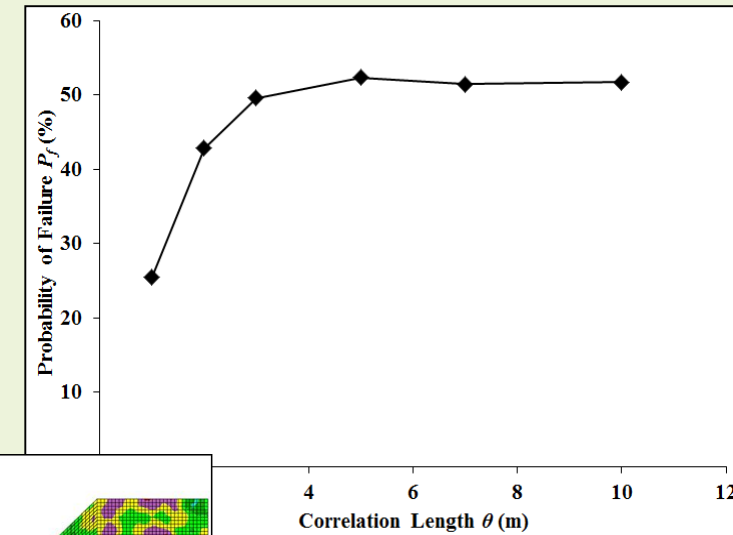
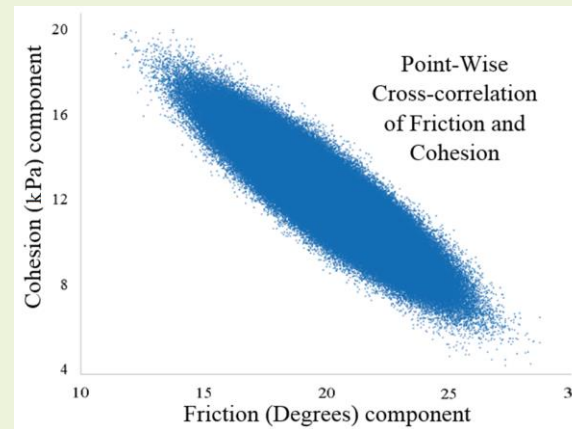
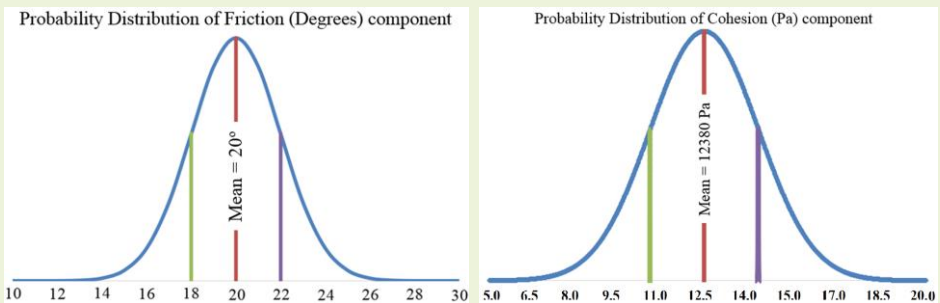
- 50 mm/day
- 100 mm/day
- 150 mm/day
- 200 mm/day
- 250 mm/day



Rainfall Induced Landslide Hazard of Guwahati City

- Local-scale probabilistic slope stability analysis

❖ Application of Random Field for soil parameters for catering uncertainty in soil parameters

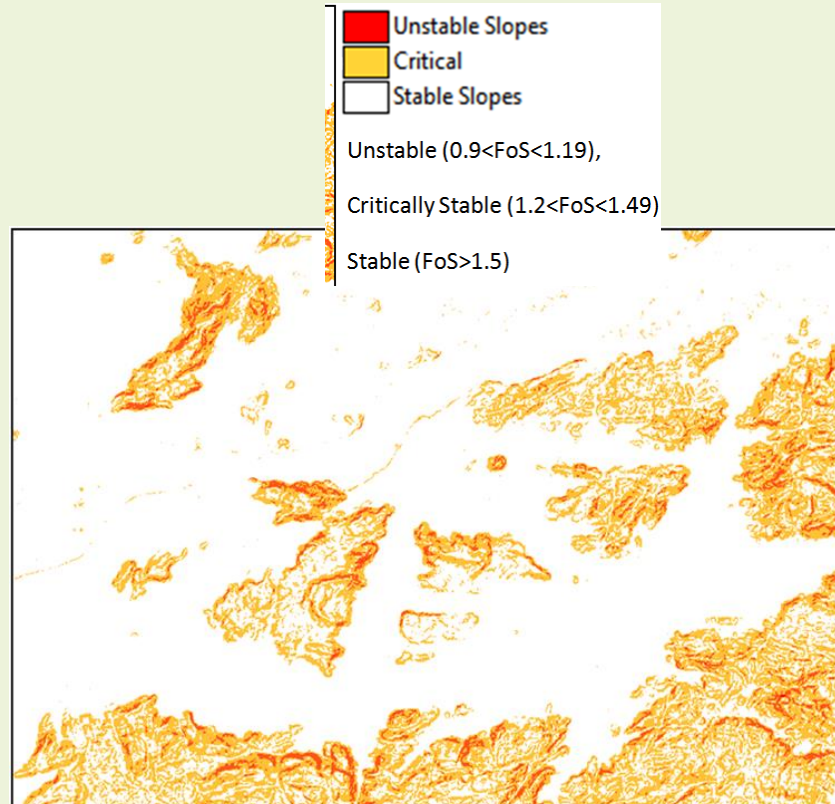
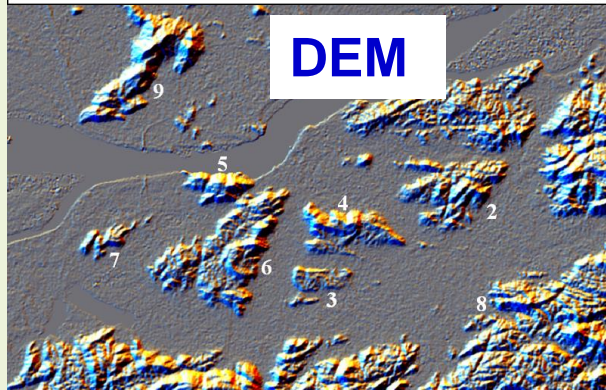
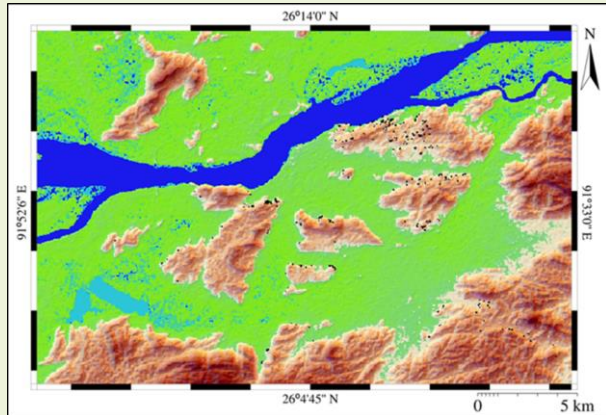


Landslide Susceptibility and LHZ of Guwahati City

Regional Scale Stability Analysis

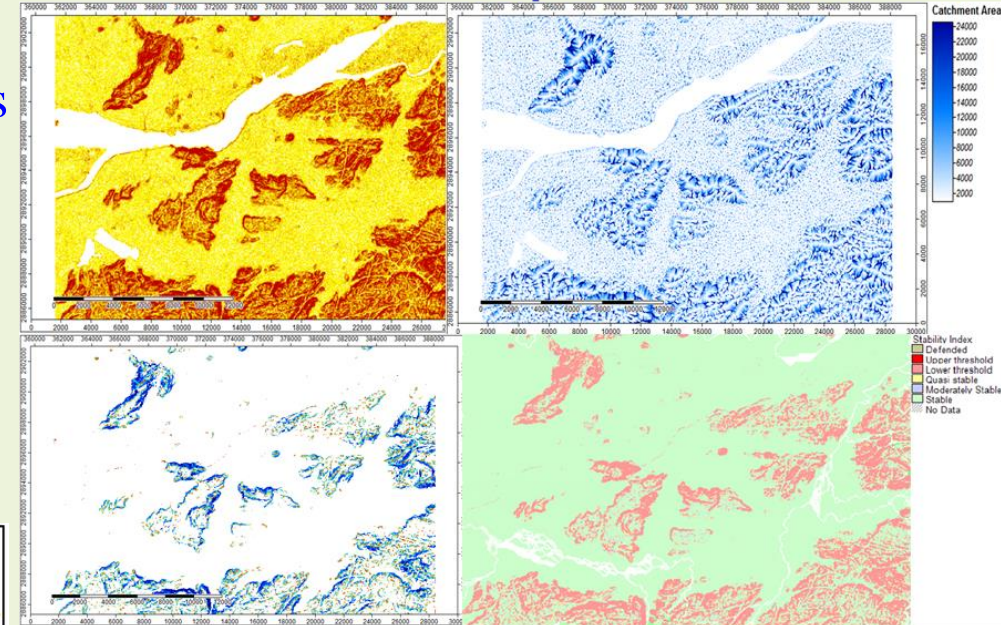
Landslide Hazard Zonation and Landslide Susceptibility Studies

- SHALSTAB, TRIGGRS, SINMAP, Physically Based Models
- GIS platform for Digital Elevation Models (DEM)



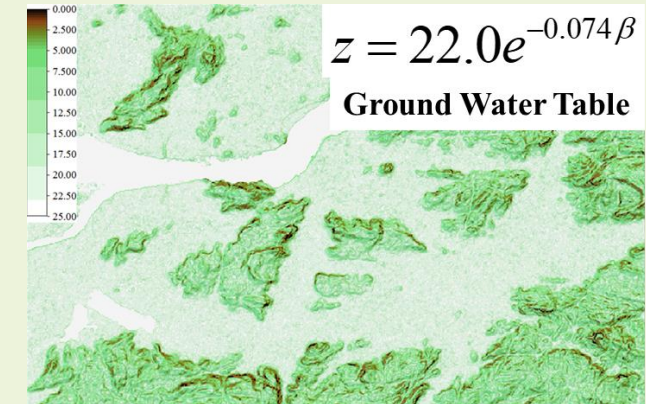
Slope Map

Map of Catchment Area



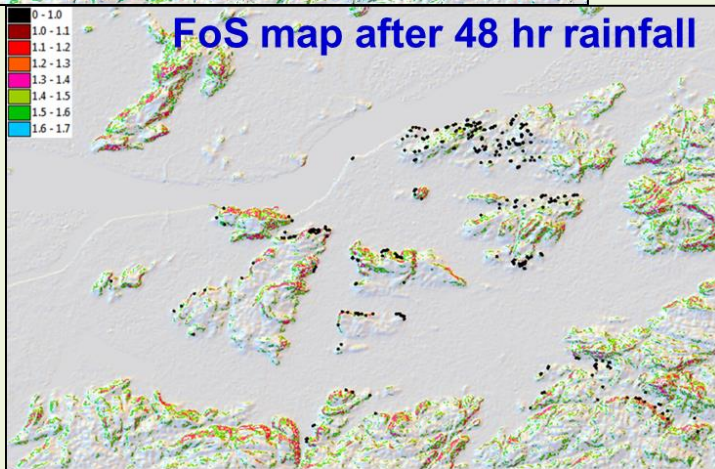
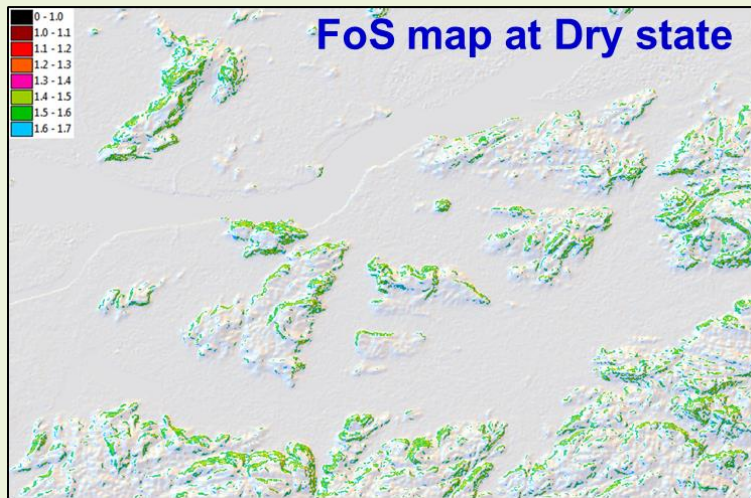
Steady-State Recharge Map

Map of Stability Index

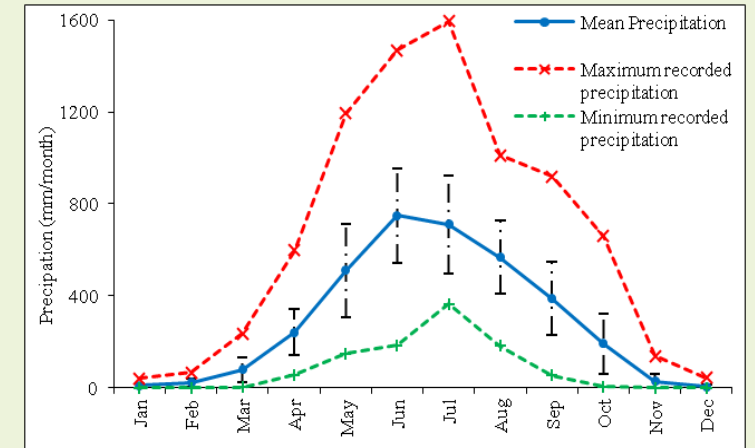
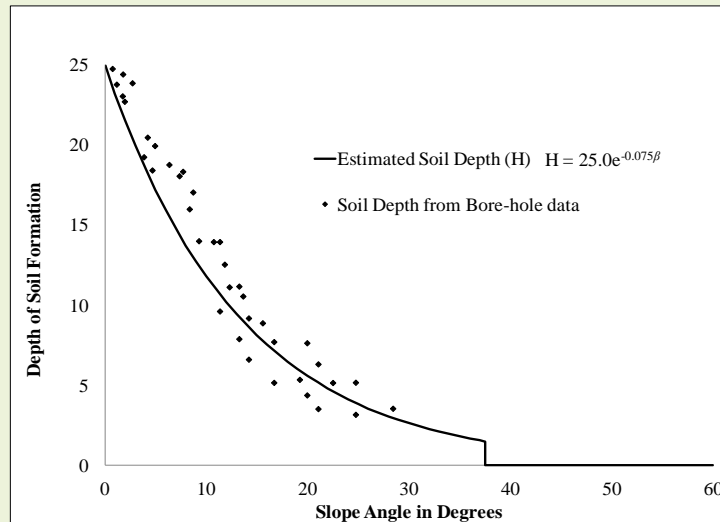


Rainfall Induced Landslide in Guwahati Region

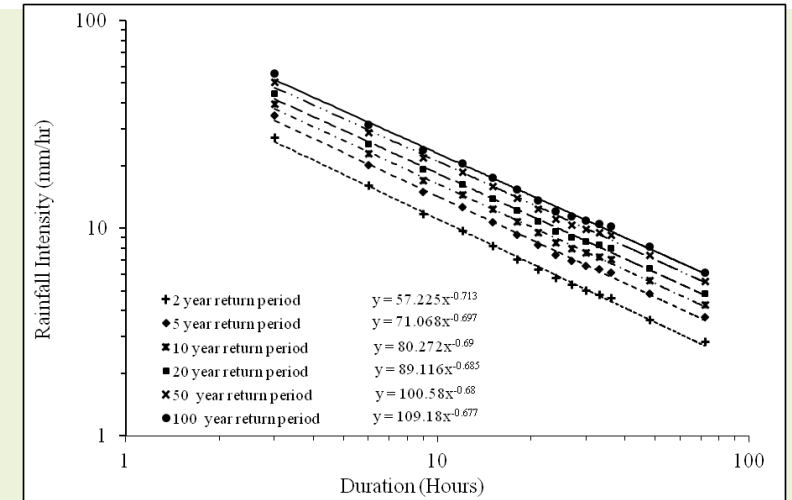
- Regional Scale Landslide Hazard Analysis
 - Incorporation of variability in rainfall and soil depth



Variation of weathered soil thickness and slope angle

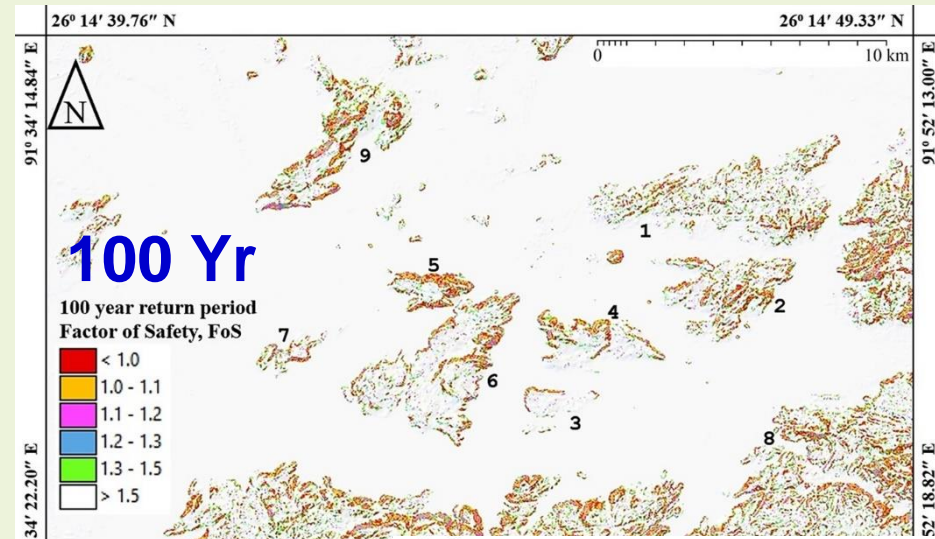
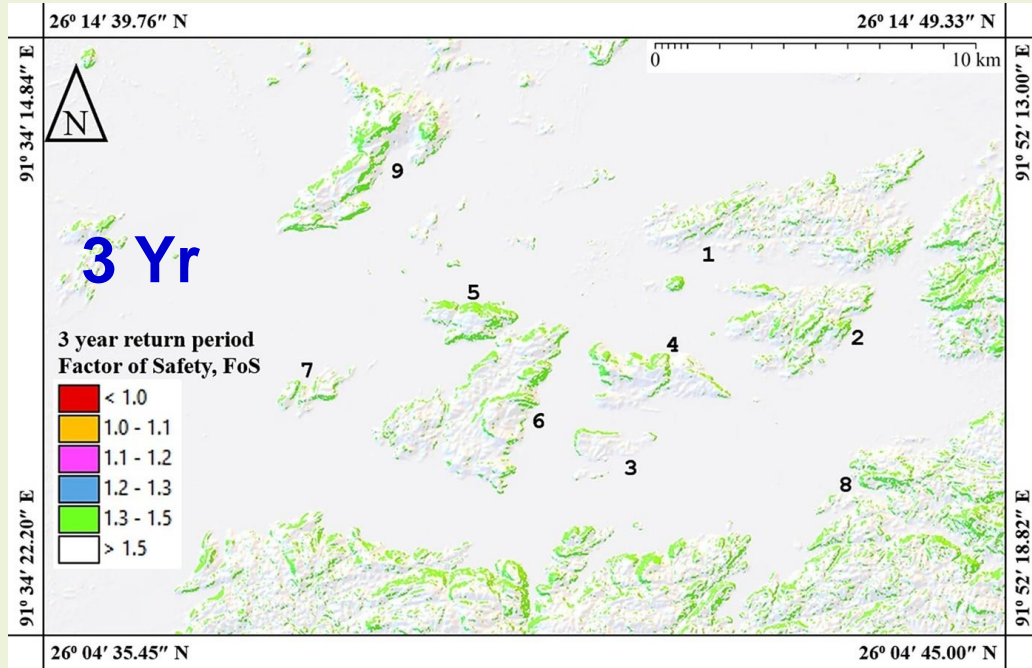


Monthly mean rainfall based on 100 years data (1901-2002) at Kamrup metropolitan

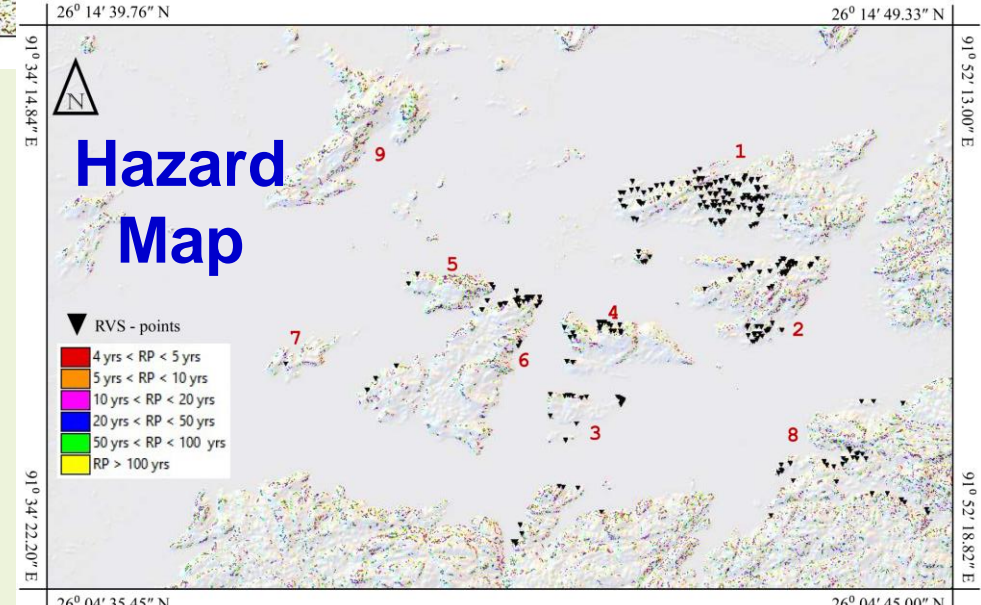
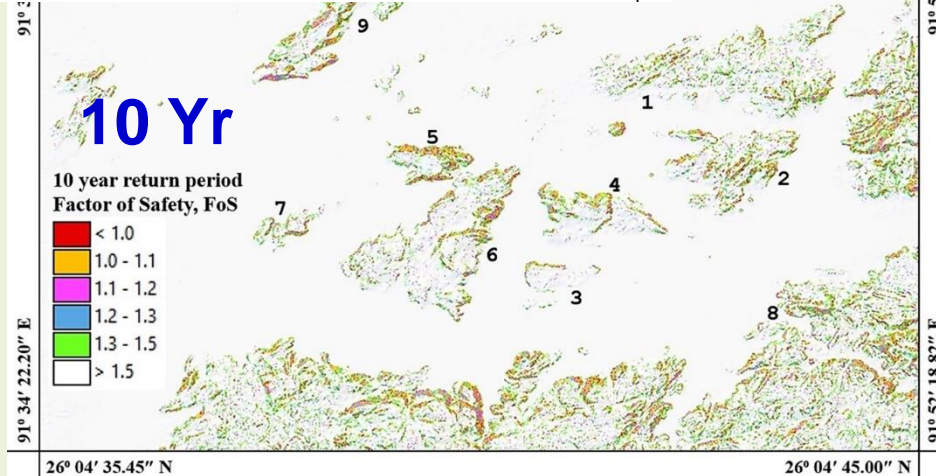


IDF curves for Guwahati region

Landslide Hazard Map of Guwahati City

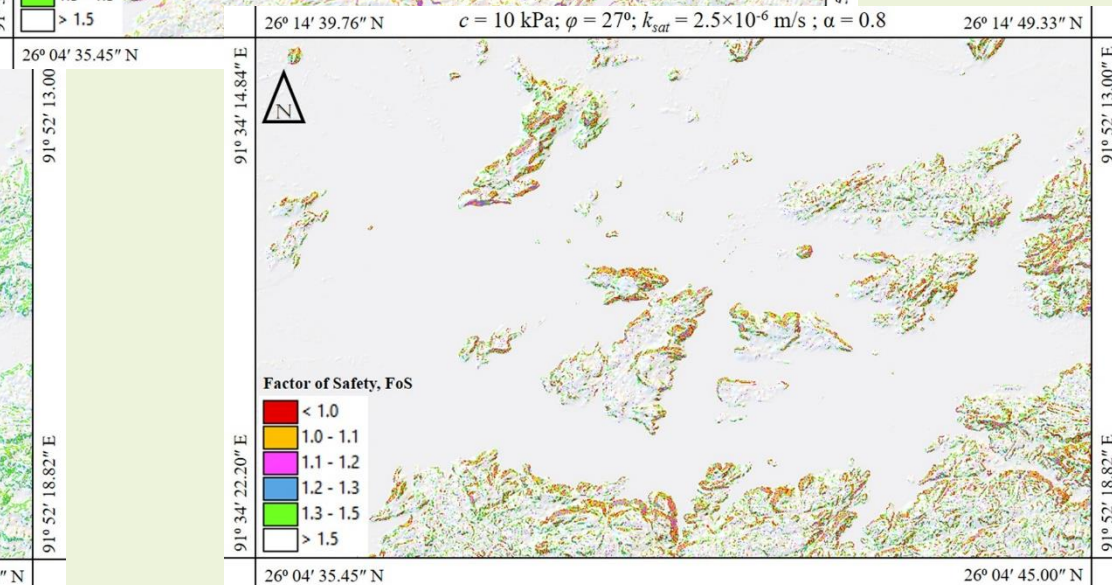
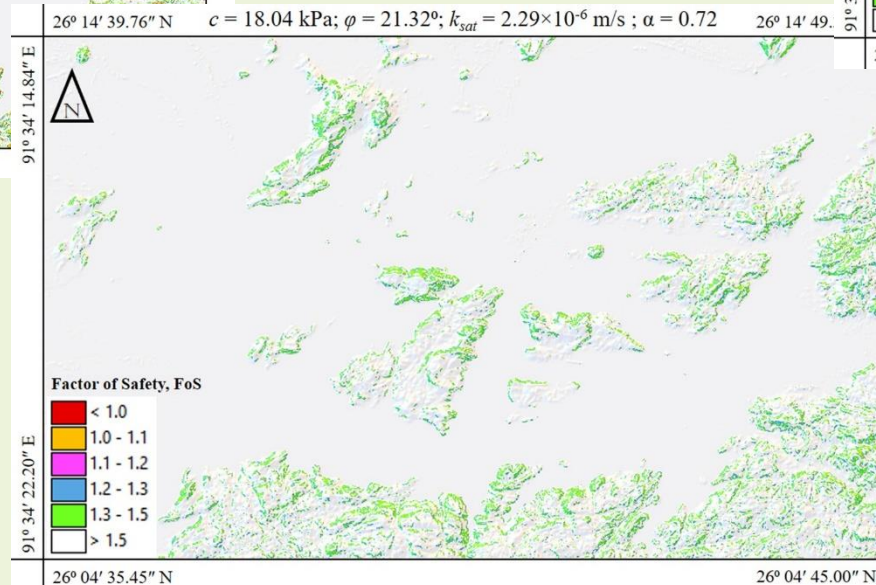
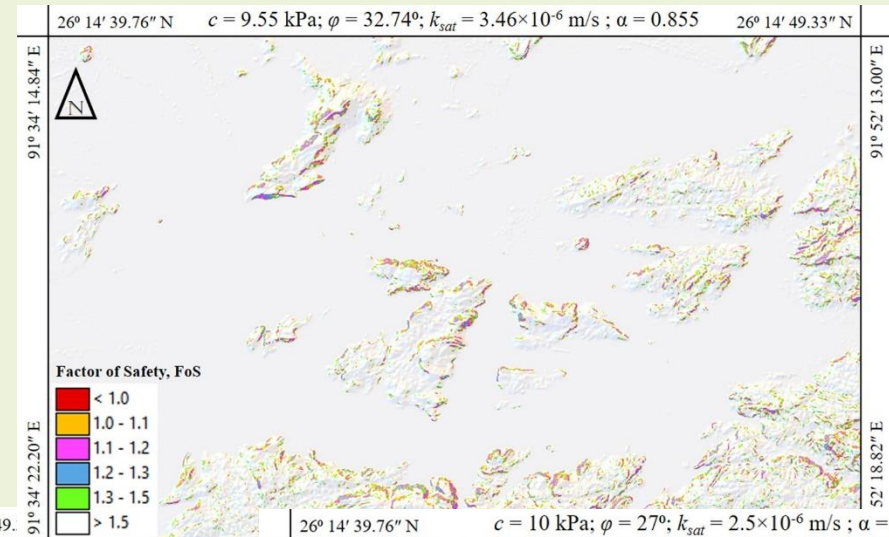
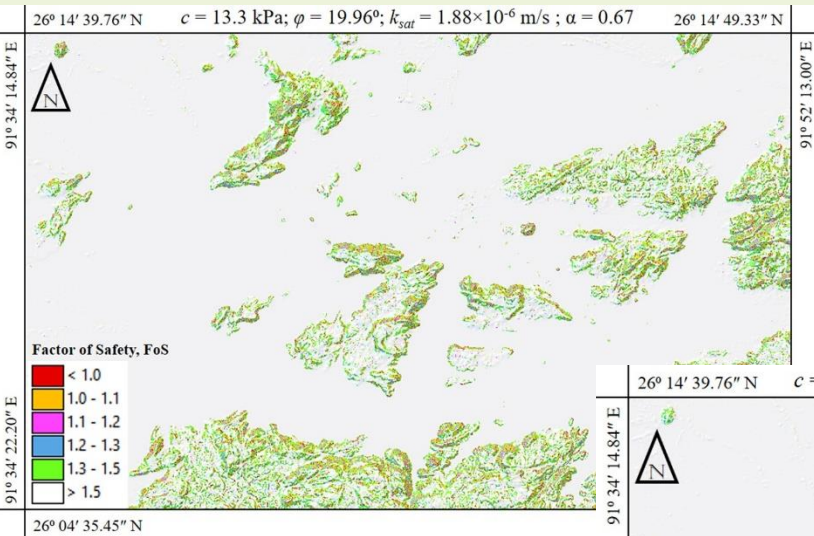


**Effect of
Return
Periods**

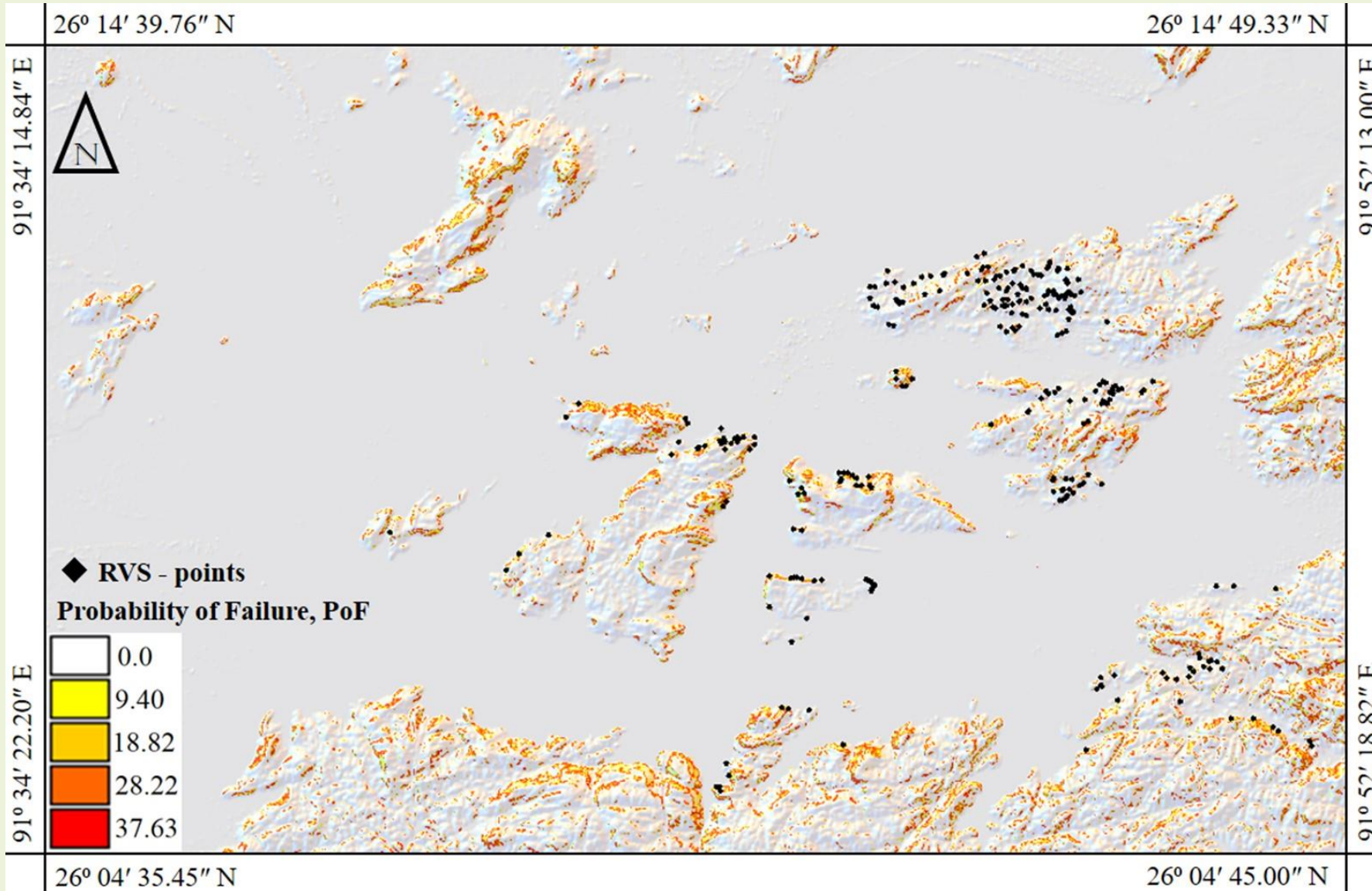


Probabilistic Landslide Hazard Analysis of Guwahati City

- Influence of uncertainty of soil parameters
 - ❖ For different mean value of soil strength parameters



Probability of Failure (PoF) Map of Guwahati City



Forensic Analysis of Landslide and Mitigation

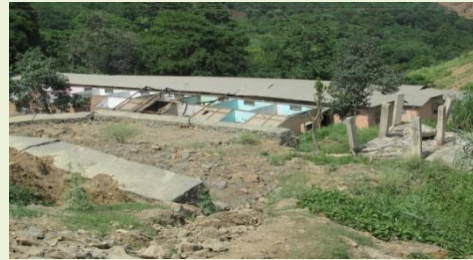
- Calcom Cement Plant, Umrangso, Assam



Priyanka

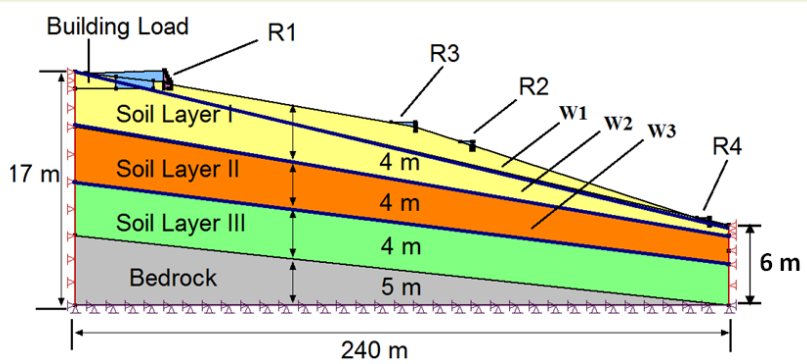
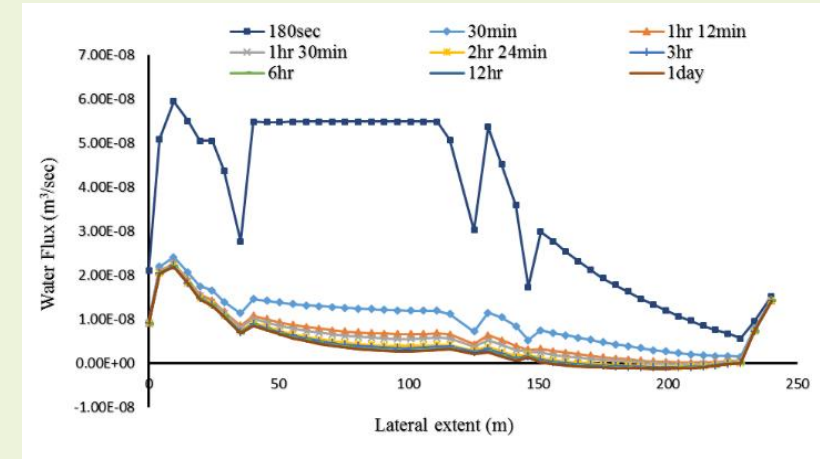


Ruplekha

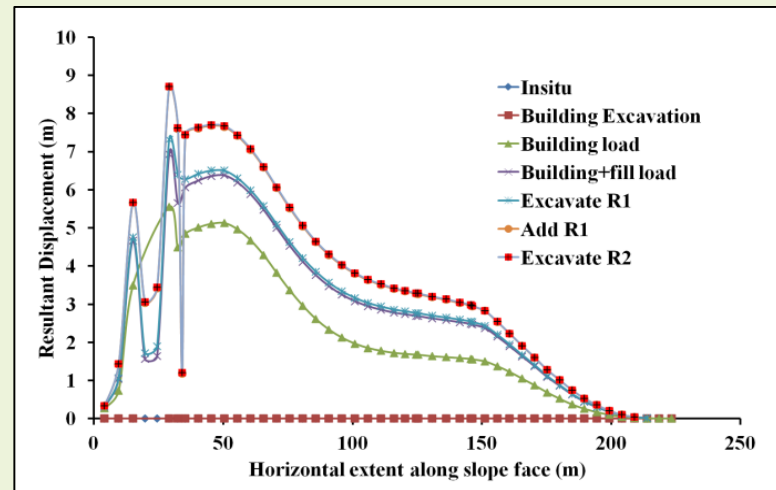
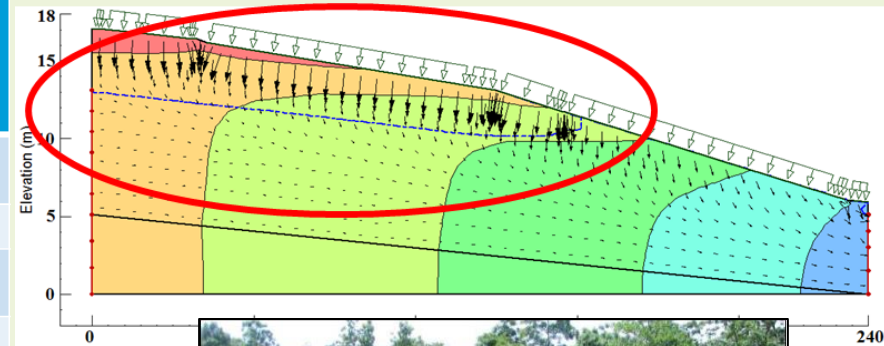


Forensic Analysis of Landslide and Mitigation

- Approximate model based on available nearby borehole data
 - ❖ Sequential construction simulated in FE model
 - ❖ Water Table depth at failure location was unknown

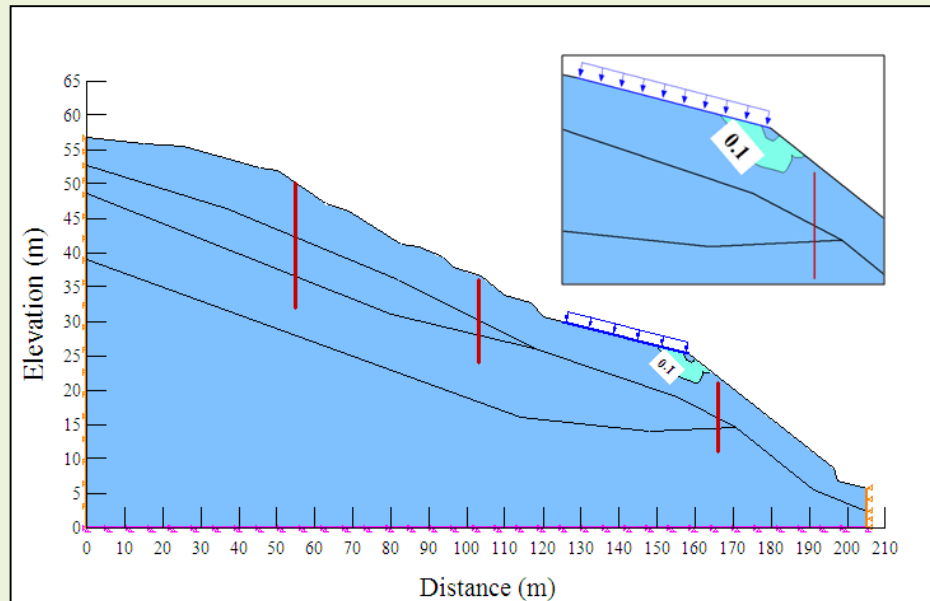
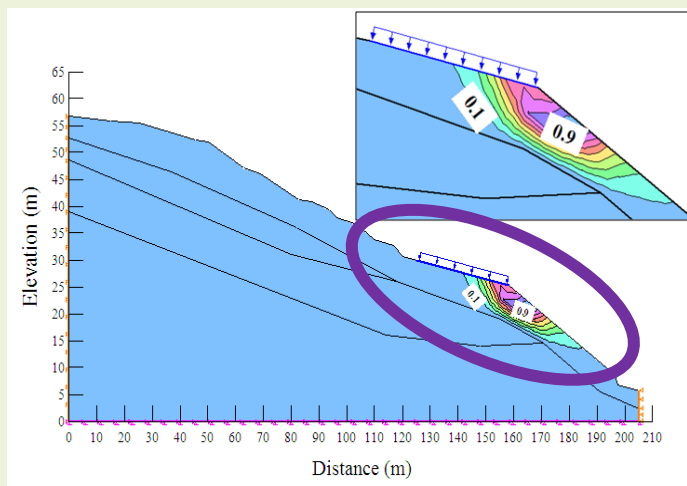
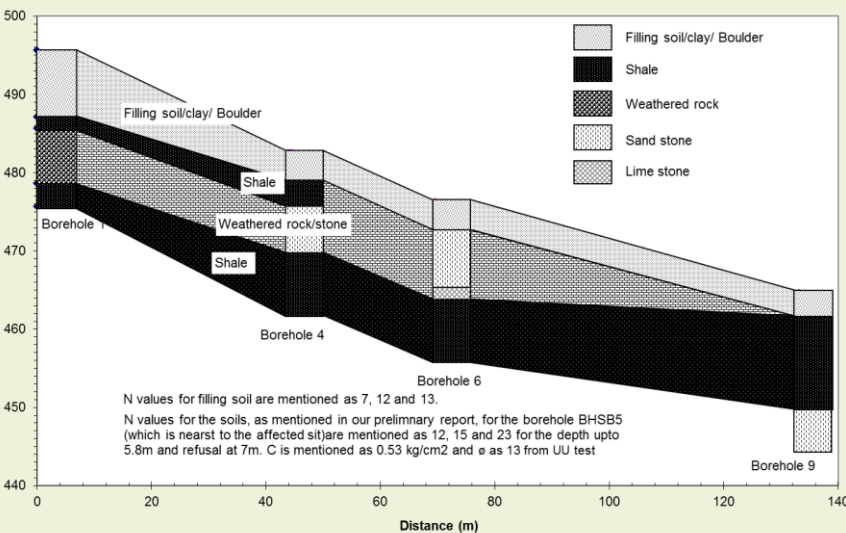
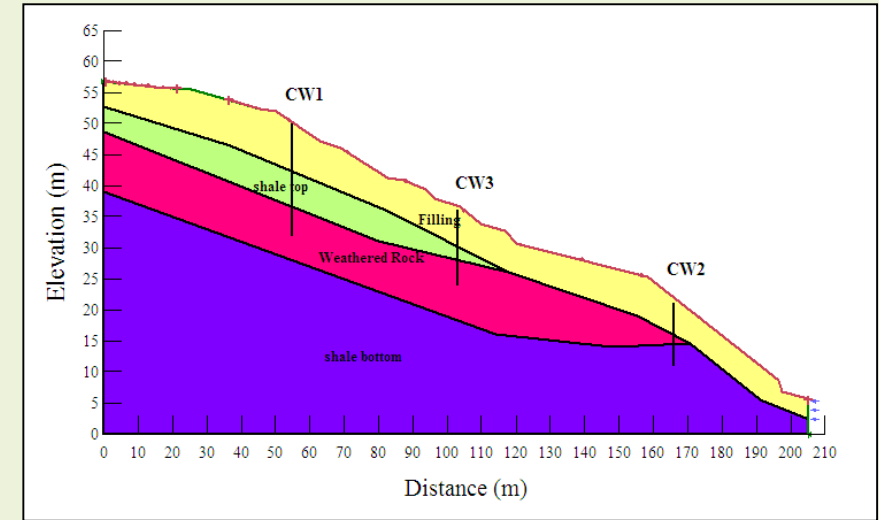
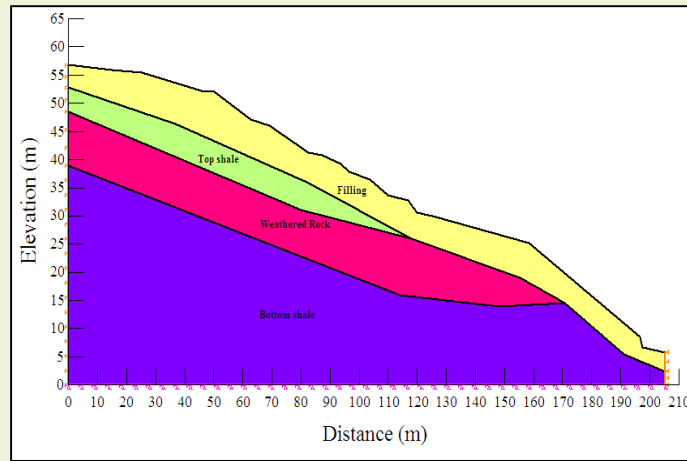
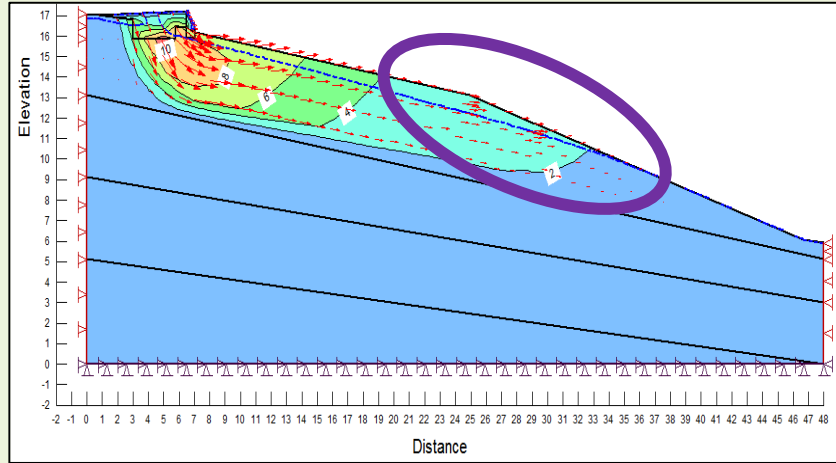


Sl. No.	Stage of construction	Dry	Water level at a ht. of 17m (W ₁)	Water level at a ht. of 13m (W ₂)	Water level at a ht. of 9m (W ₃)
		FoS Values	FoS Values	FoS Values	FoS Values
1	In-situ	2.112	1.411	1.588	1.511
2	Building foundation excavation	2.1	1.373	1.577	1.513
3	Imposition of building load	0.976	0.821	0.793	0.769
4	Filling back of foundation	0.967	0.850	0.802	0.774
5	Excavation for R1	1.015	0.875	0.825	0.805
6	Construction and backfilling of R1	0.985	0.838	0.798	0.785
7	Excavation for R2	1.373	0.817	1.065	1.025
8	Construction and backfilling of R2	1.344	0.752	0.967	1.007
9	Excavation for R3	1.288	1.029	1.035	0.975
10	Construction and backfilling of R3	1.294	1.024	0.984	0.959



Forensic Analysis of Landslide and Mitigation

- Mismatch with field displacement in approximate model



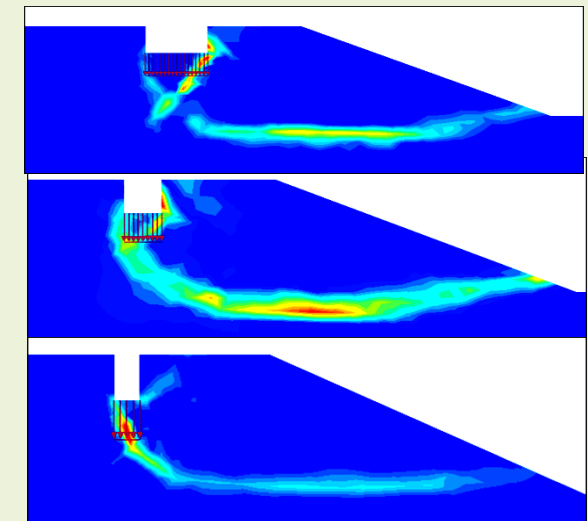
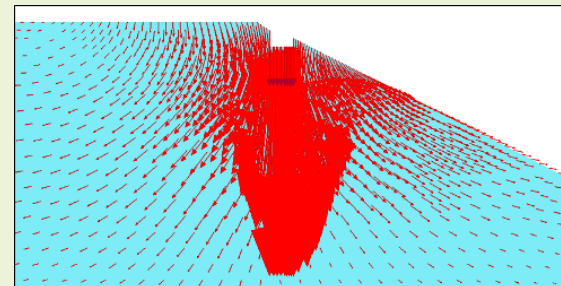
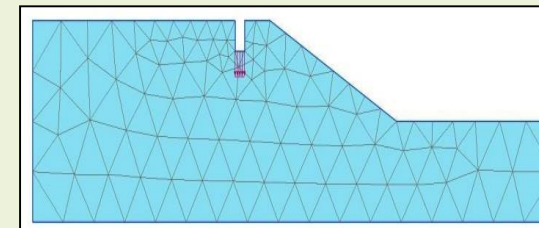
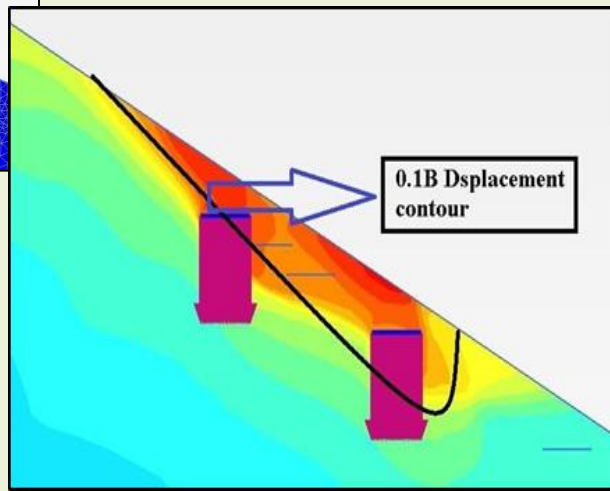
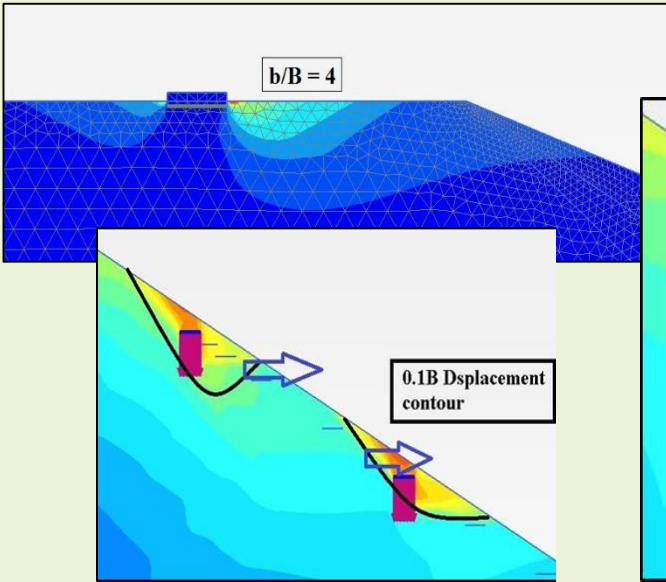
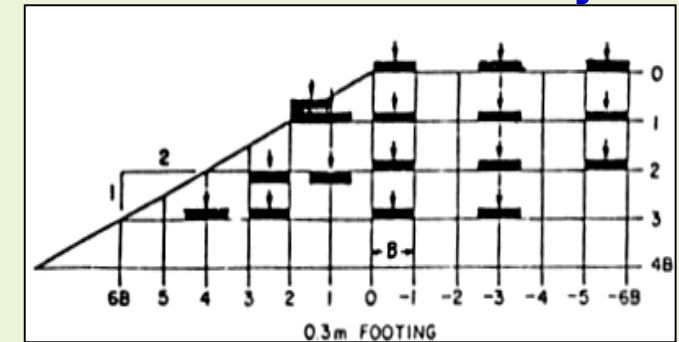
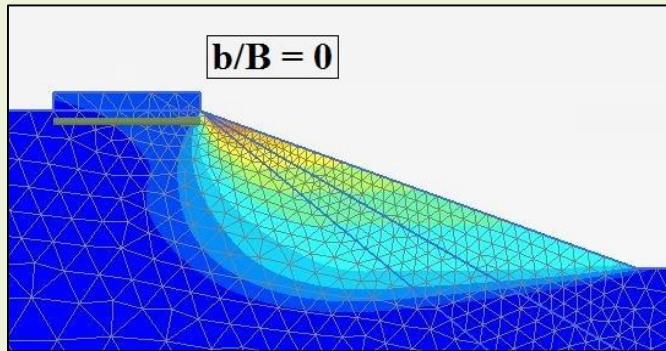
Response of Foundations on Slopes

- Static and seismic behavior of foundations on slopes
 - ❖ FE Analysis to identify the evolution of failure mechanism

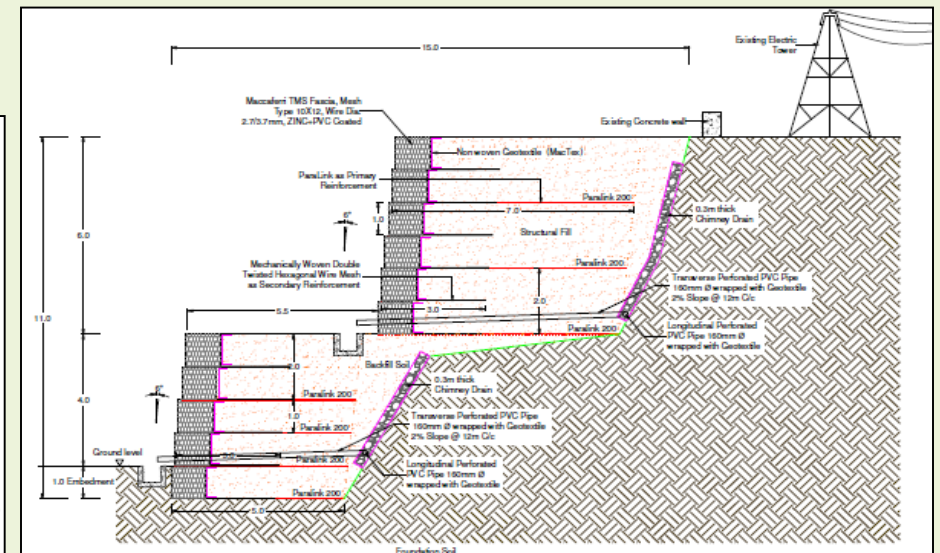
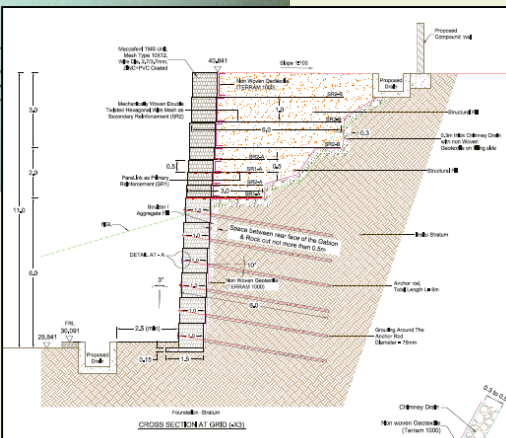
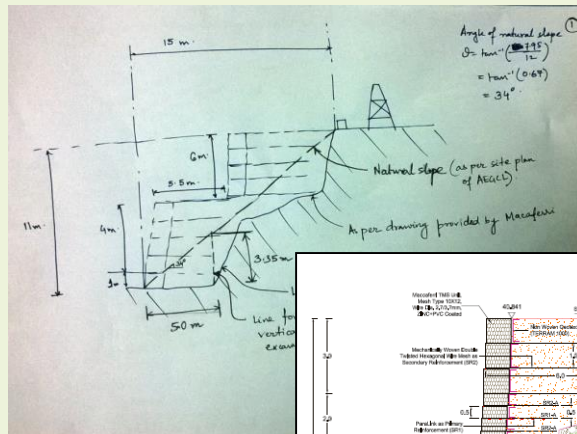
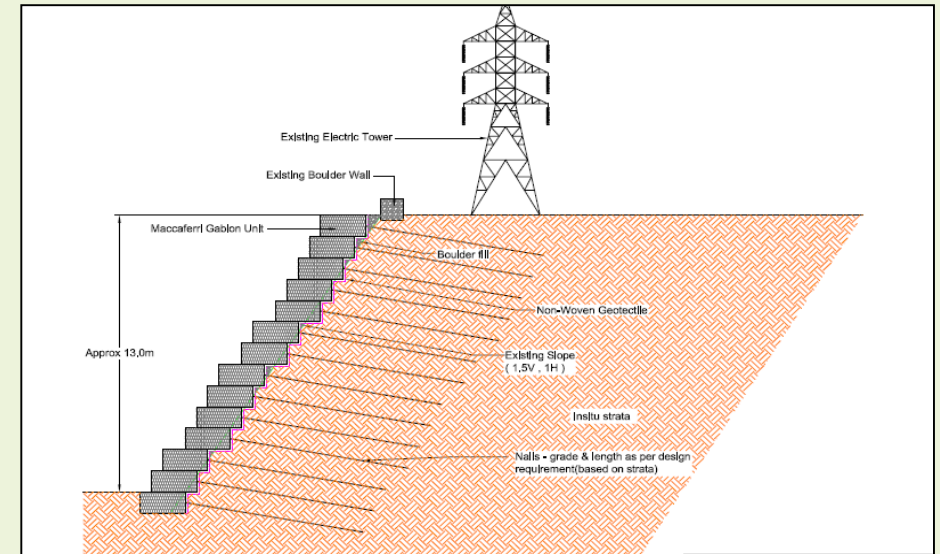
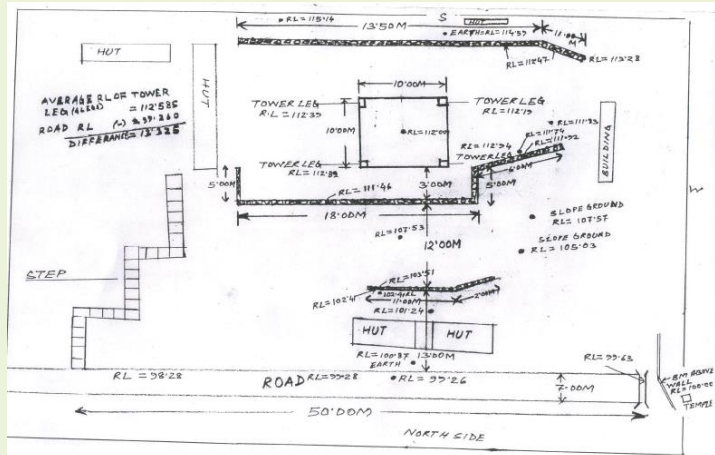


Rana

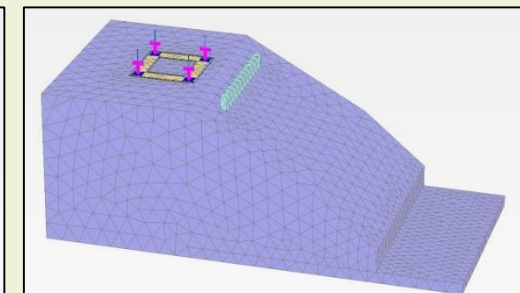
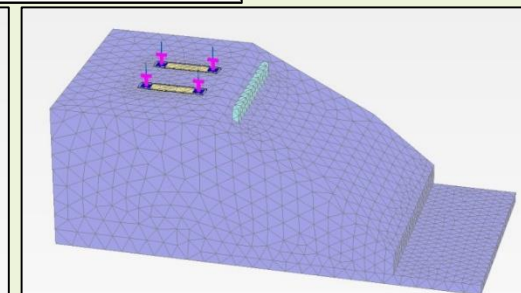
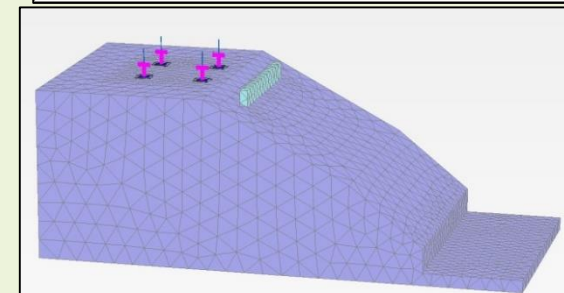
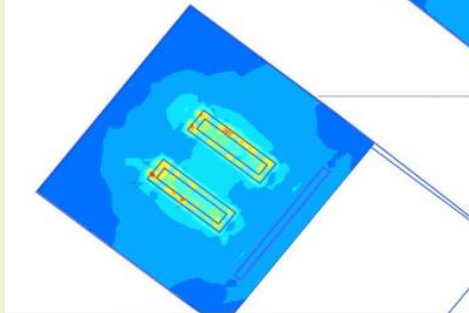
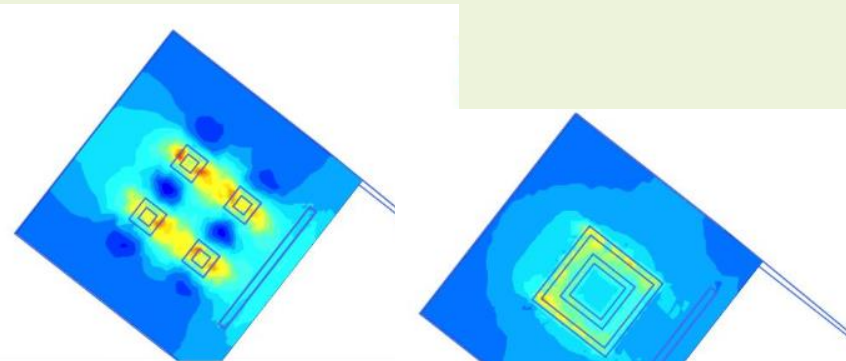
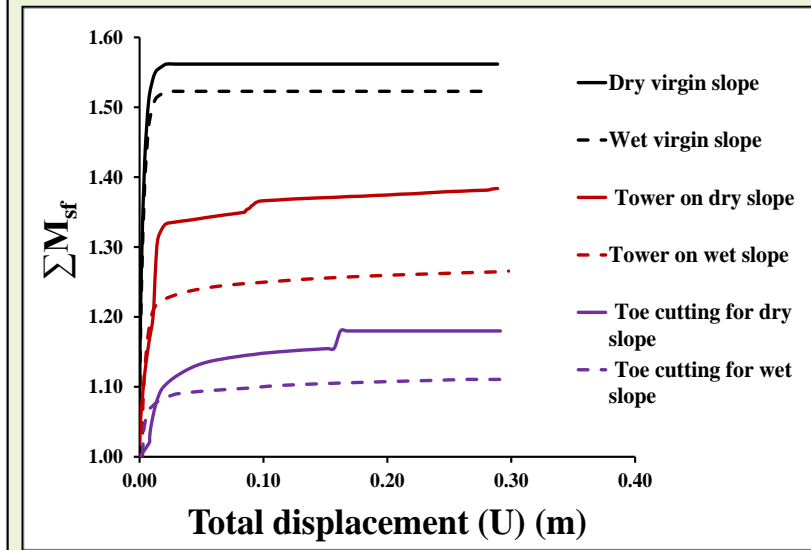
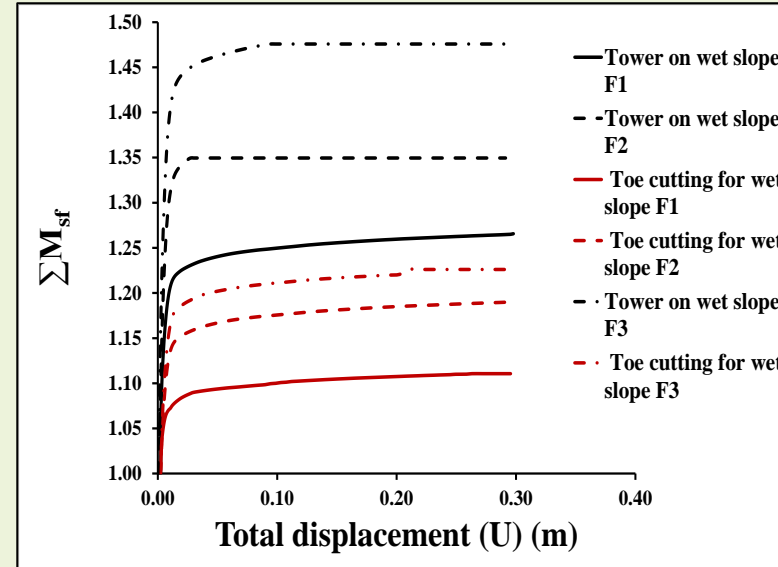
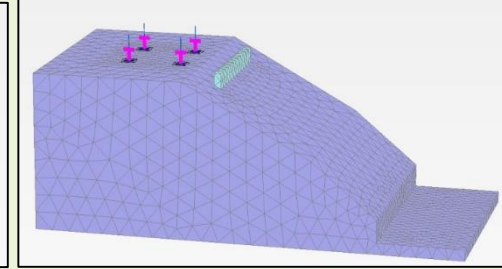
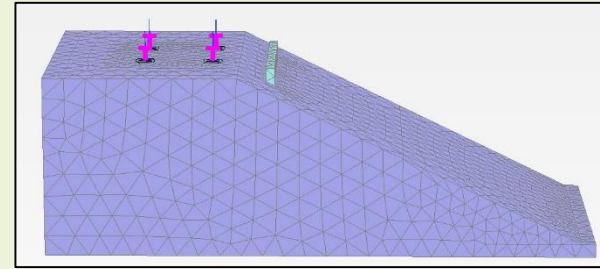
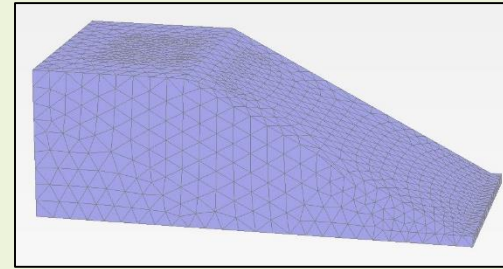
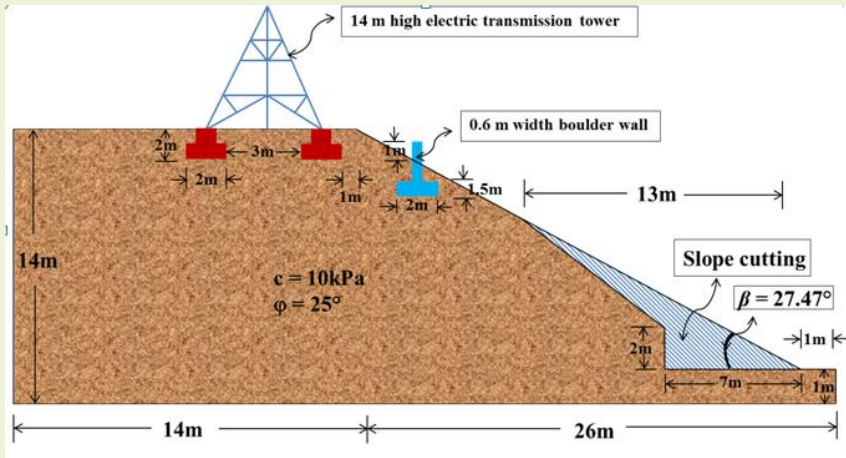
Abhijit



AEGCL Transmission Tower on Disturbed Slope, Sarusajai



AEGCL Transmission Tower on Disturbed Slope, Sarusajai

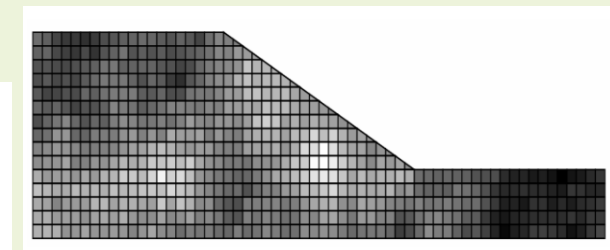
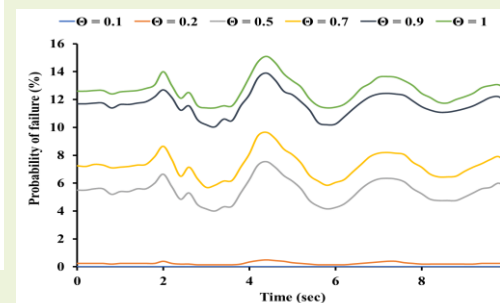
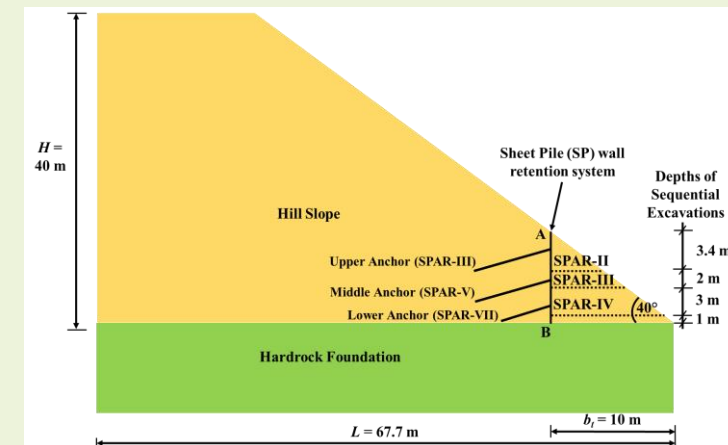
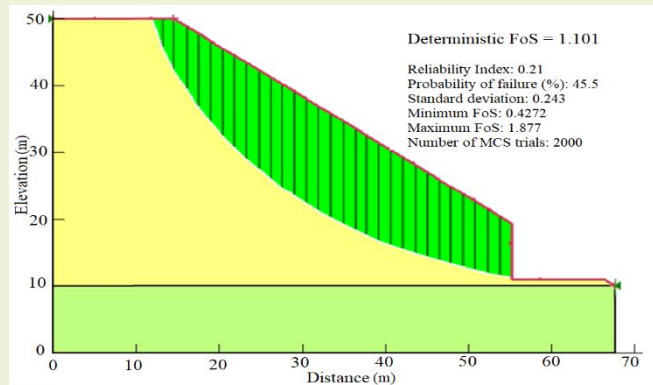
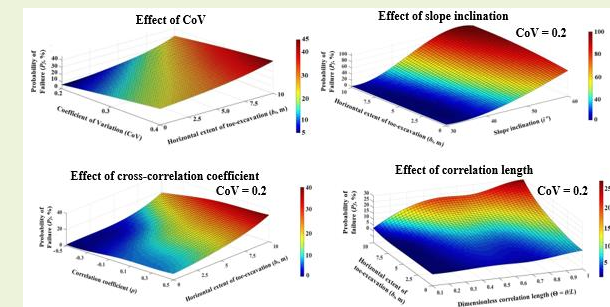
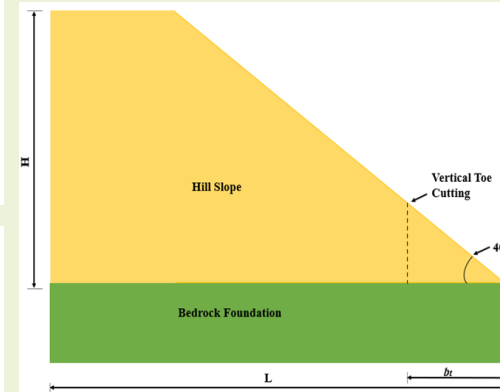
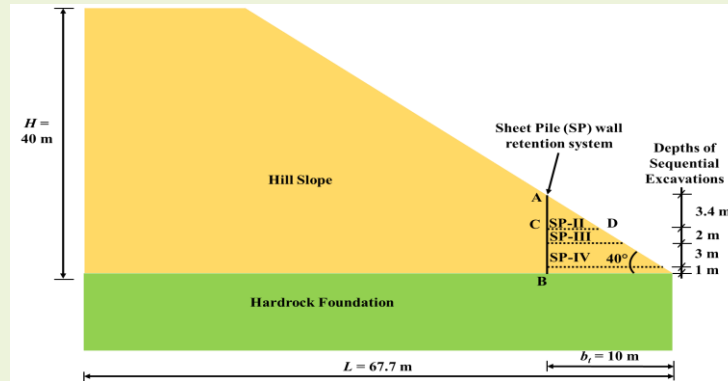
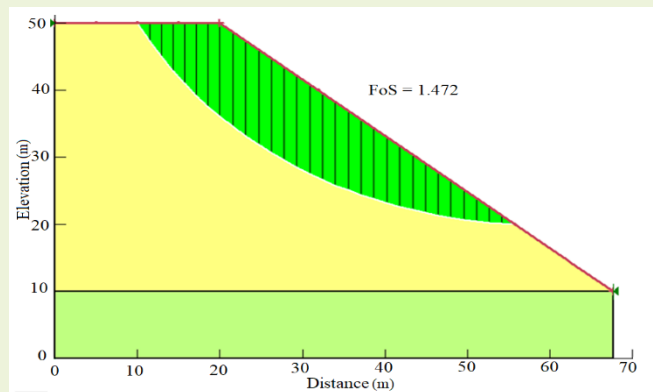


Probabilistic Stability Assessment of Toe-Excavated Hillslopes

- Consideration of uncertainties and variabilities
 - ❖ Slope stability assessment and mitigation
 - ❖ Random FE and Probabilistic assessment

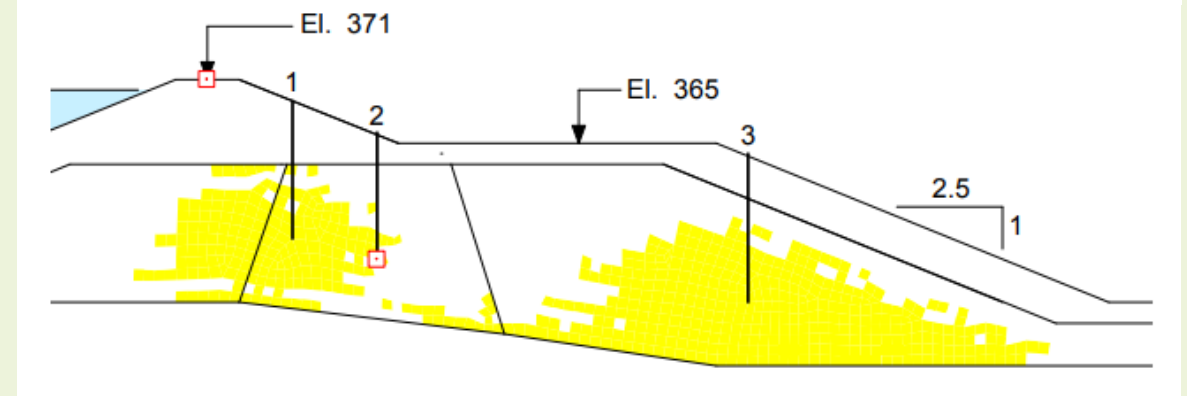
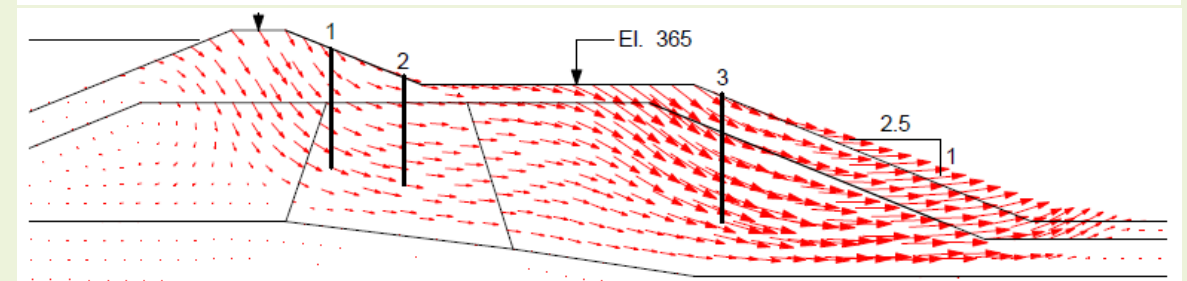
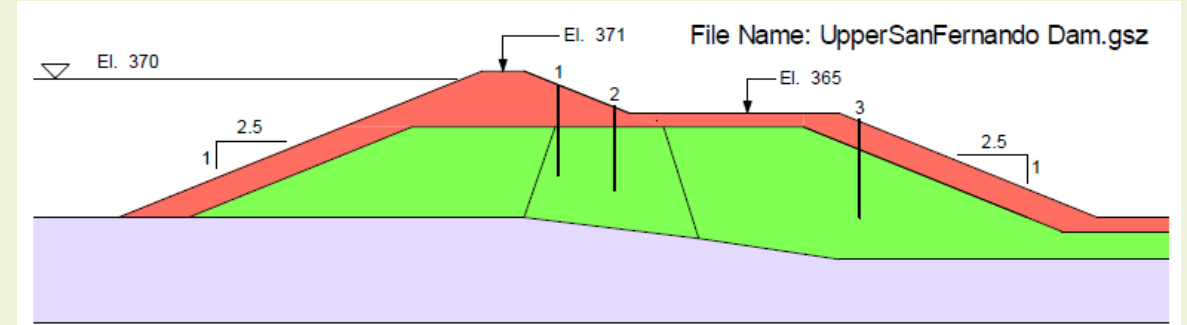
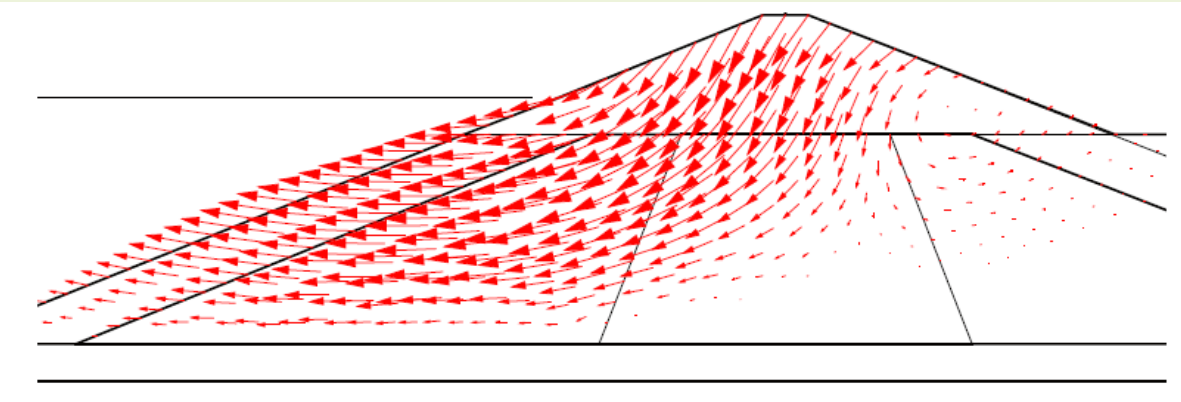
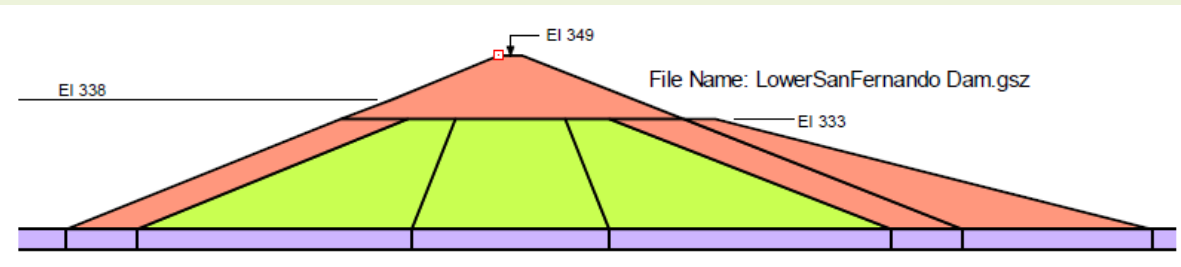


Rubi



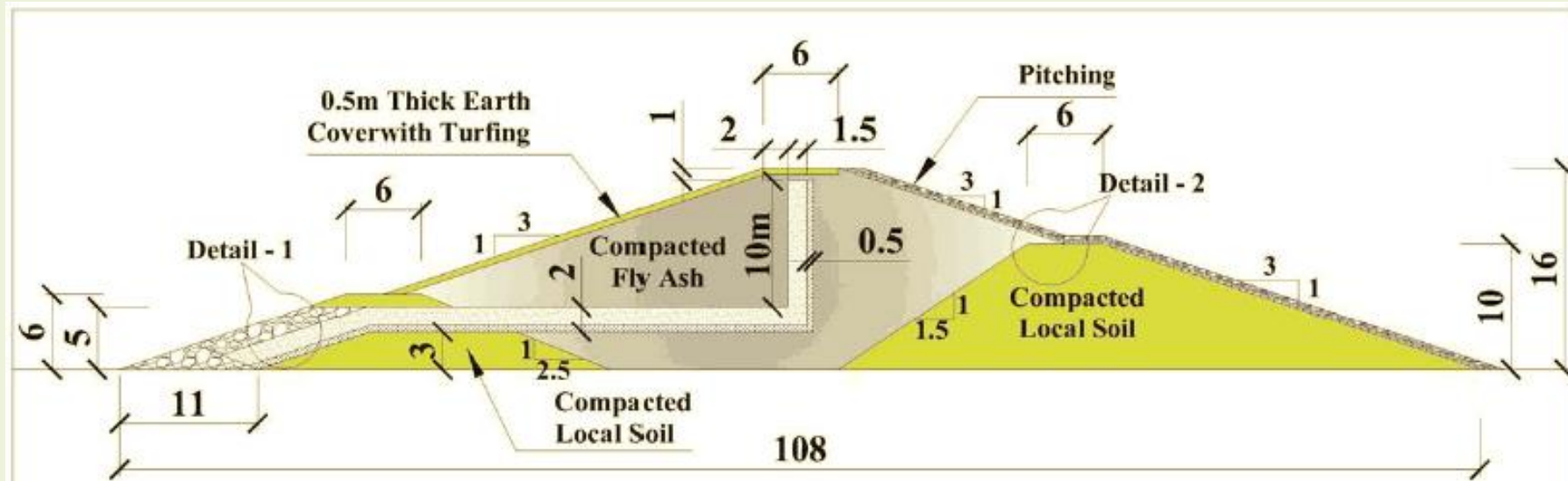
Geotechnical Engineering of Earthen Dams and Embankments

- Curious case of San Fernando Dam Failure due to 1971 San Fernando Earthquake

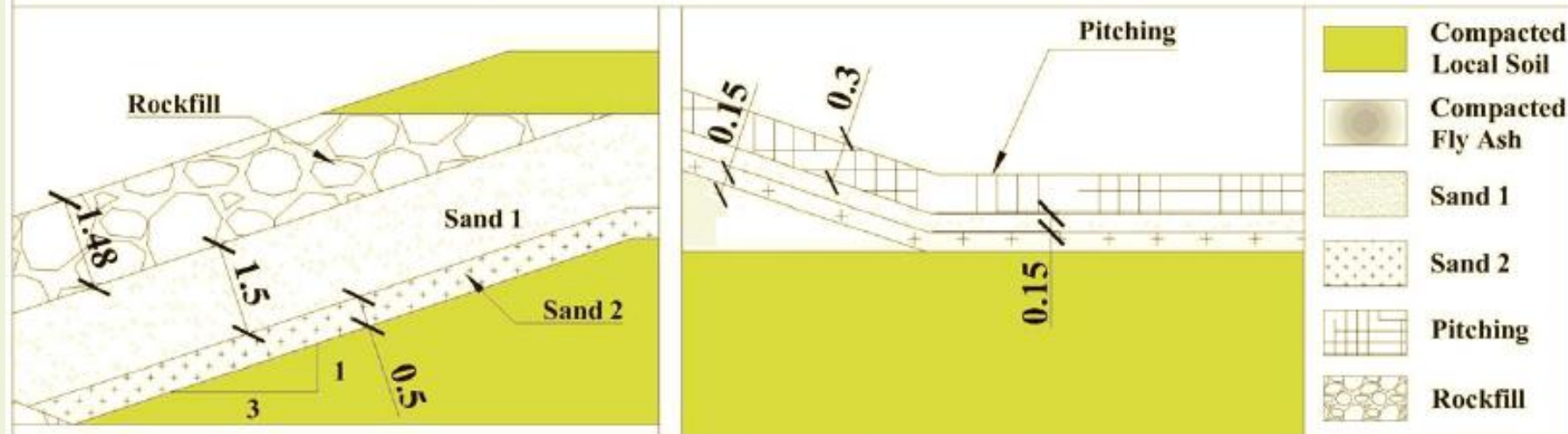


Proper drainage is absolute necessity

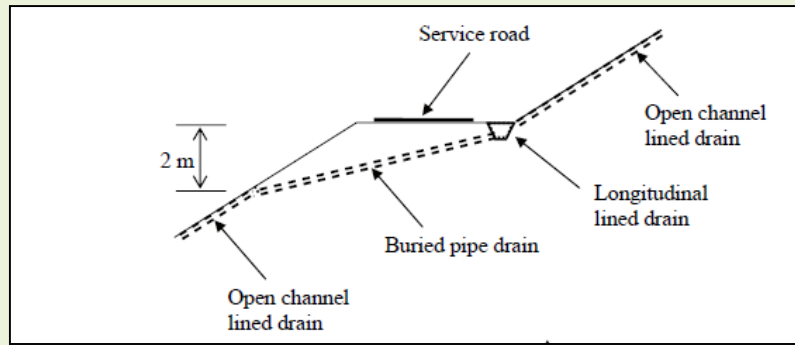
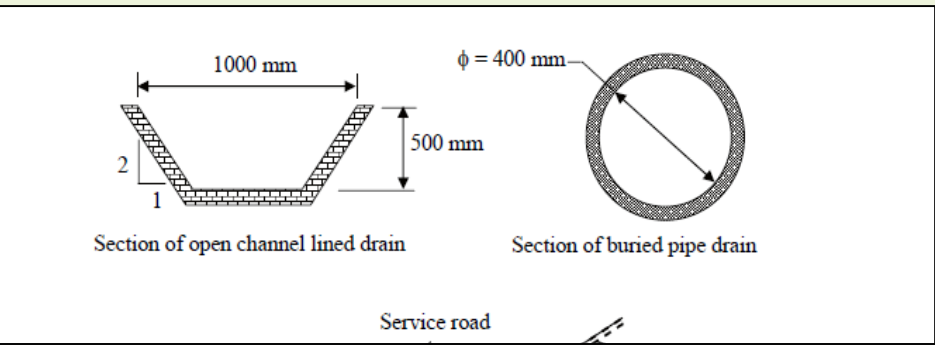
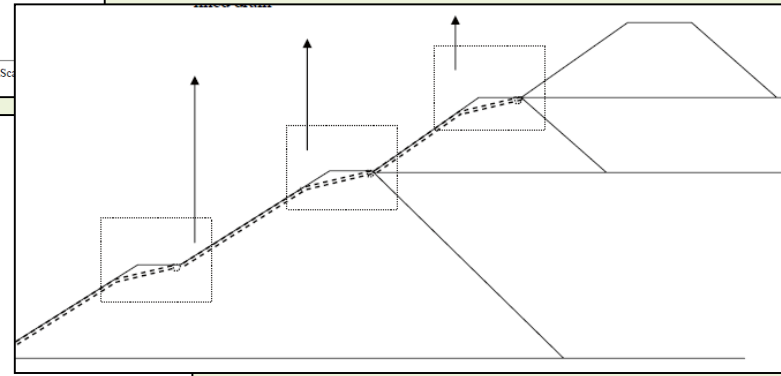
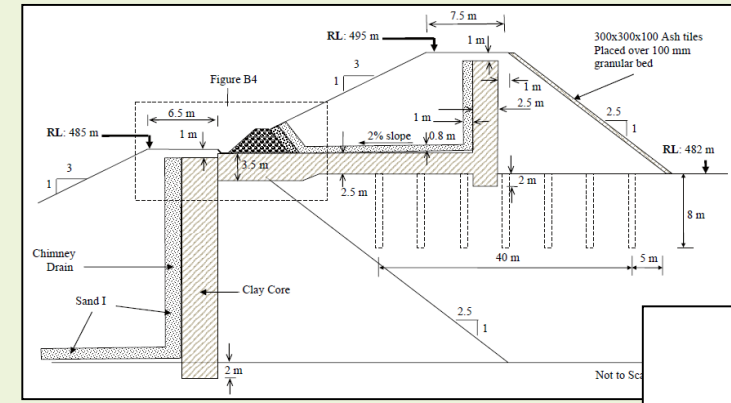
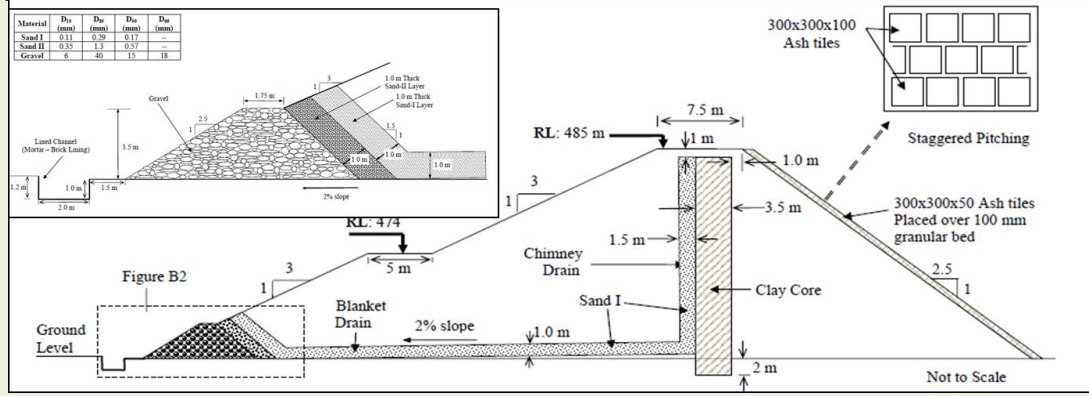
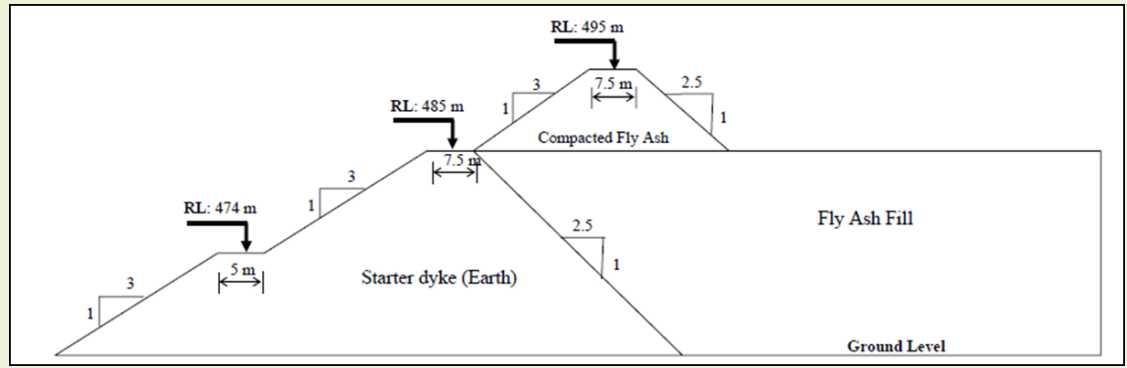
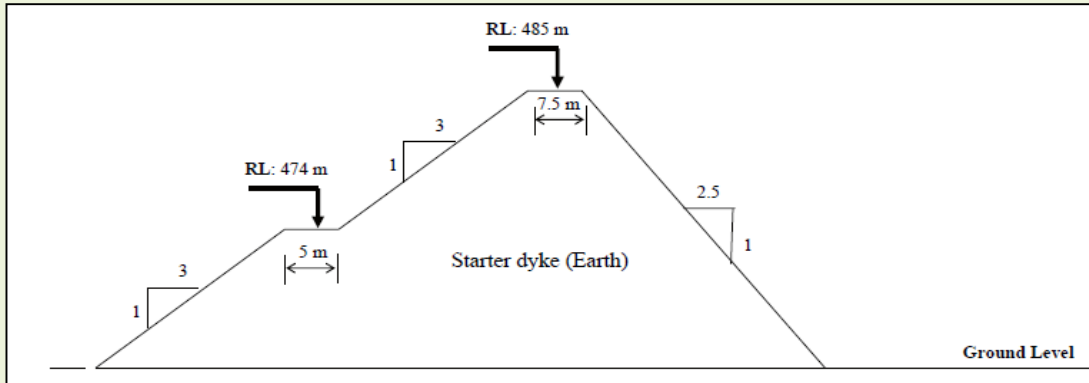
Stability of Ash Dyke, NTPC, Bongaigaon, Assam



TYPICAL CROSS-SECTION OF MAIN ASH DYKE



Ash Dyke, NTPC, Birsinghpur, MP

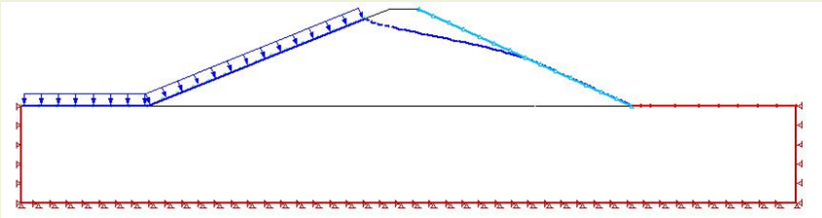
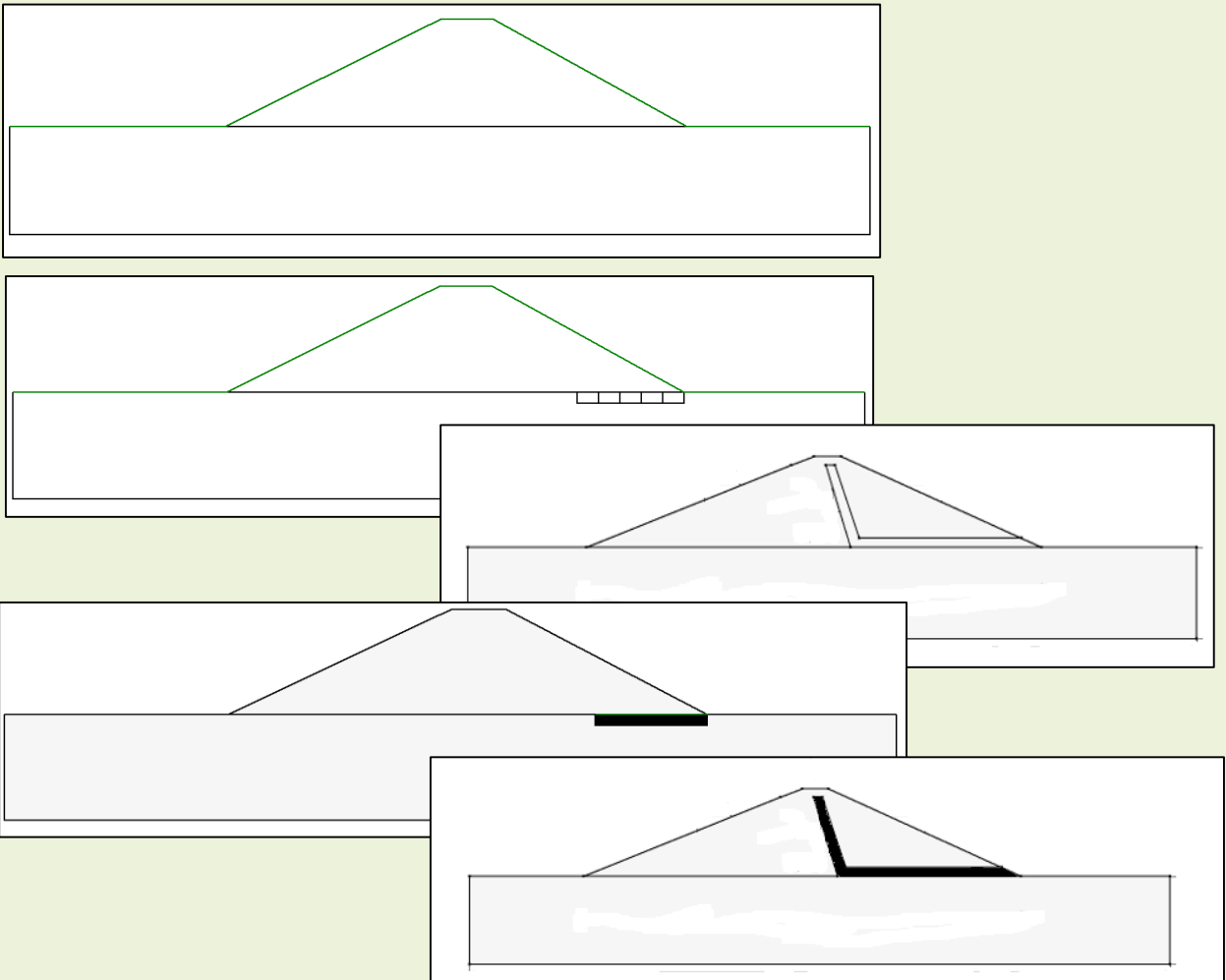


Clogging of Drainage Blanket of an Earthen Dam/Dyke

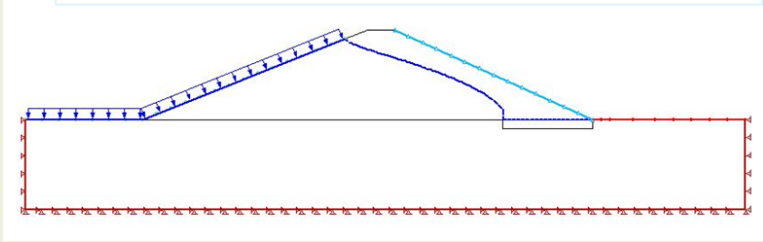
- Clogging of drainage blanket jeopardizes the geo-hydraulic stability of earthen dam



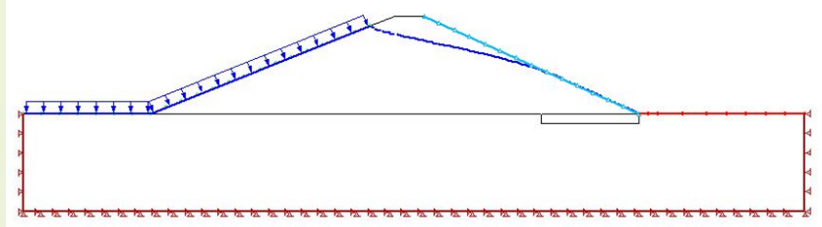
Priyanka



Without drainage blanket condition (NDC)



Fully functional drainage blanket condition (FDC)

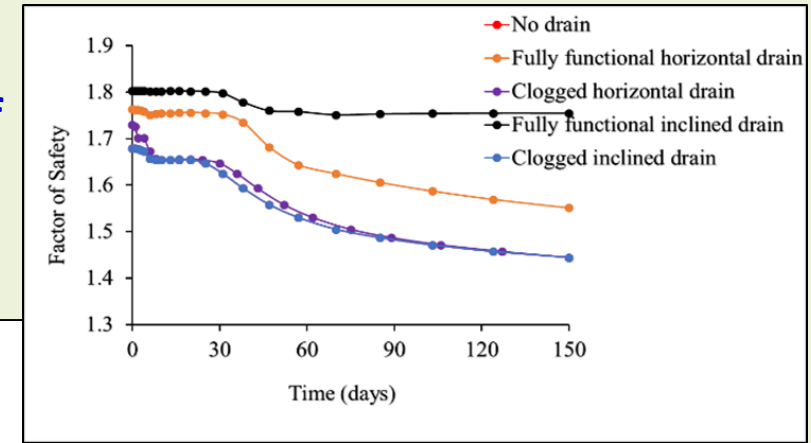


Clogged drainage blanket condition (CDC)

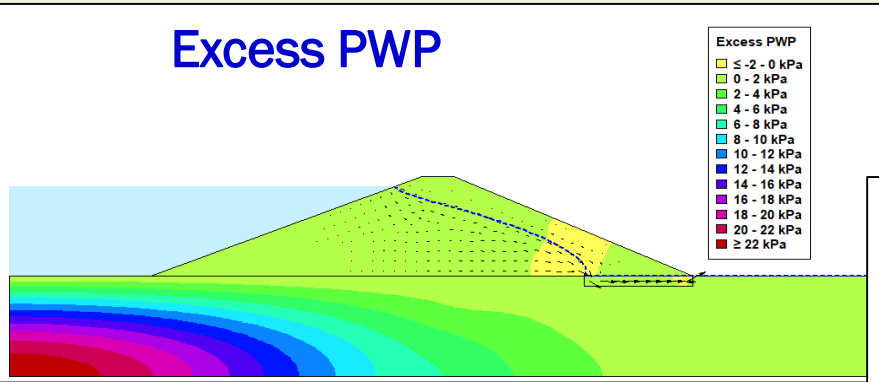
Clogging of Drainage Blanket of an Earthen Dam/Dyke

- Clogging of drainage blanket jeopardizes the geohydraulic stability of earthen dam

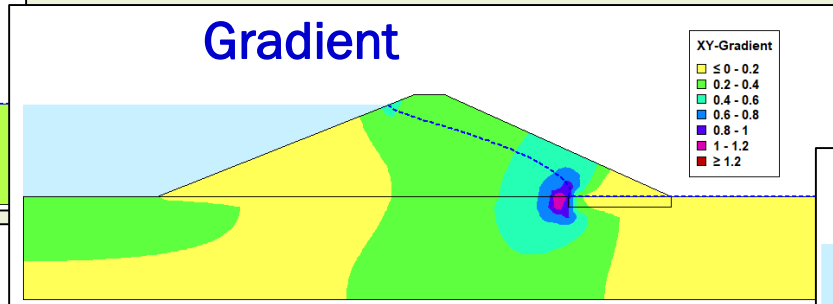
Long-term reduction of FoS of d/s face due to reservoir rise-up



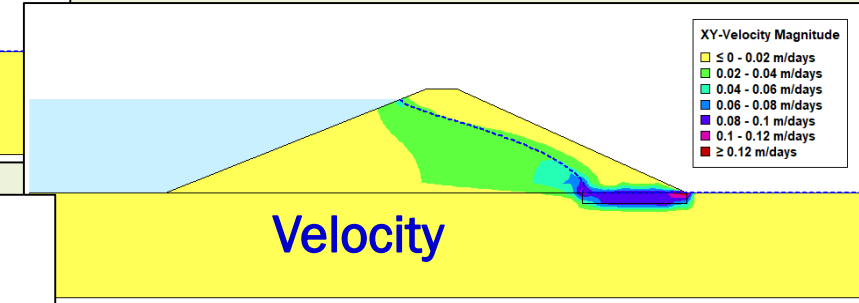
Excess PWP



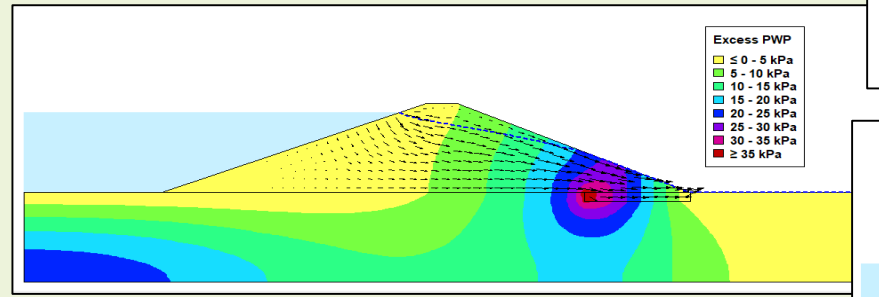
Gradient



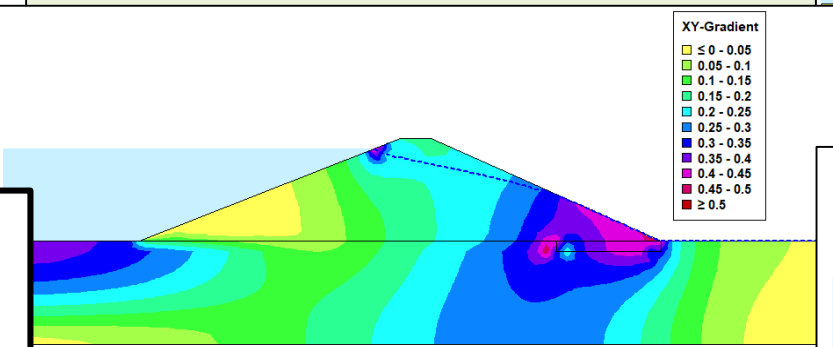
Velocity



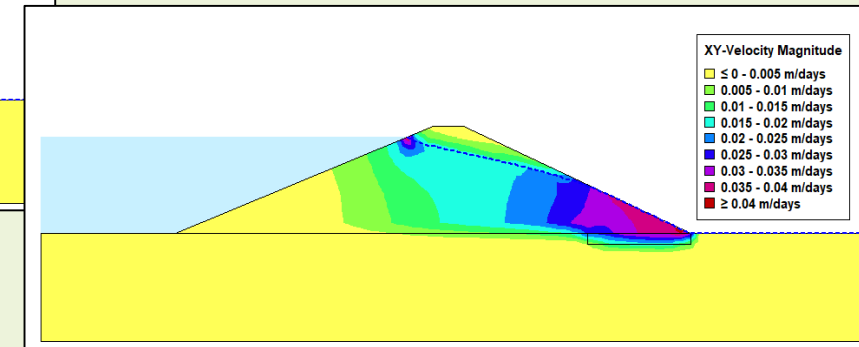
Excess PWP



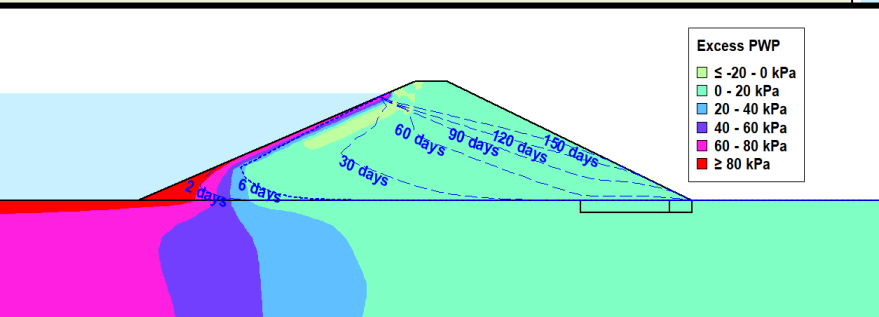
XY-Gradient



XY-Velocity Magnitude



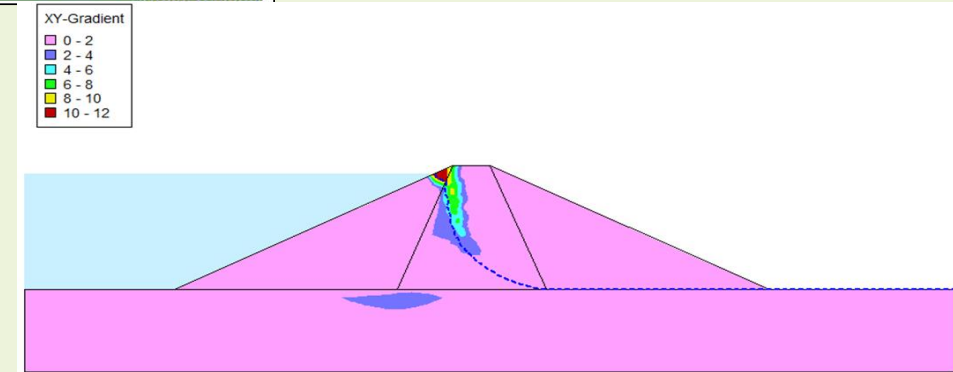
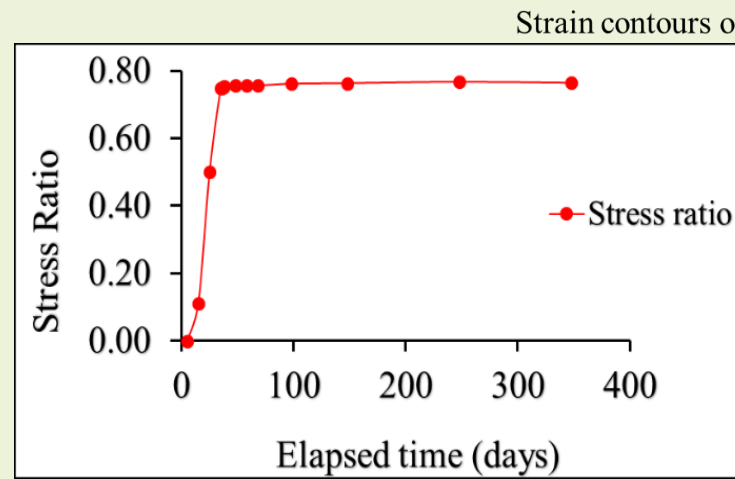
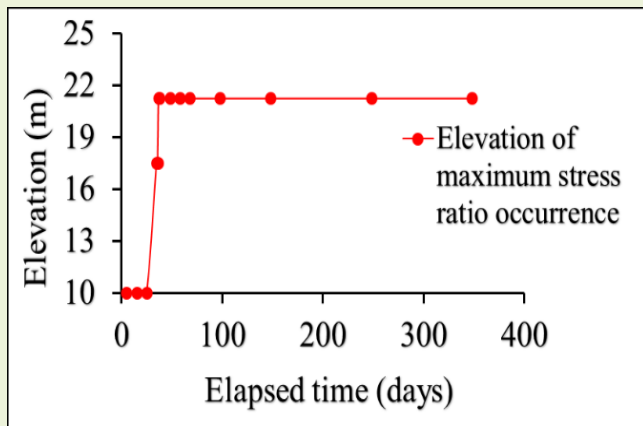
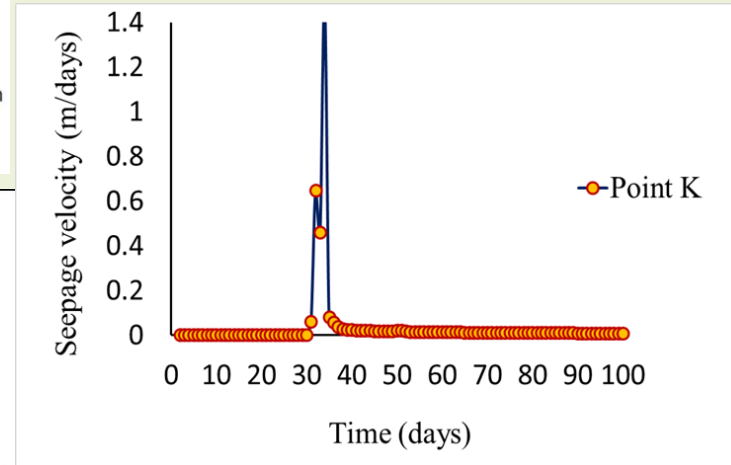
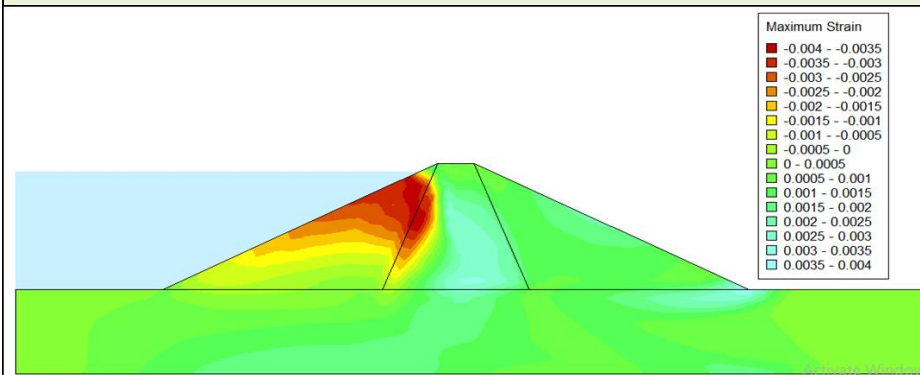
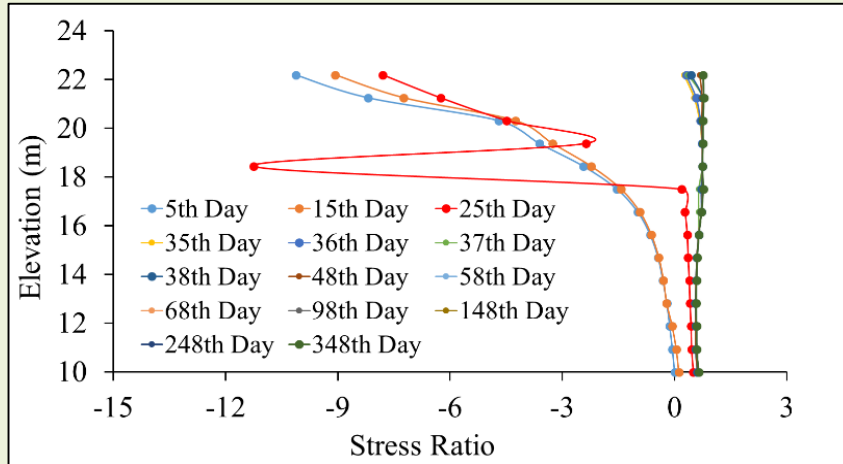
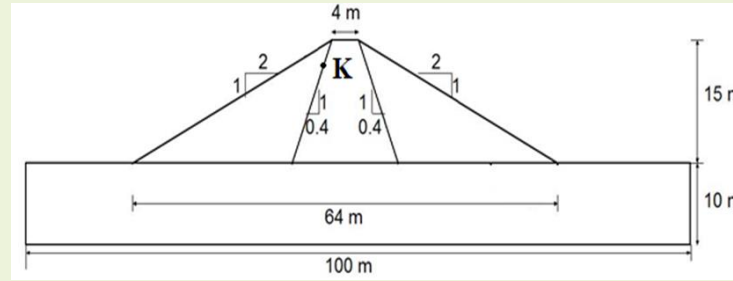
Excess PWP



Long-term rise of phreatic surface

Core-Cracking of Zoned Earthen Dam

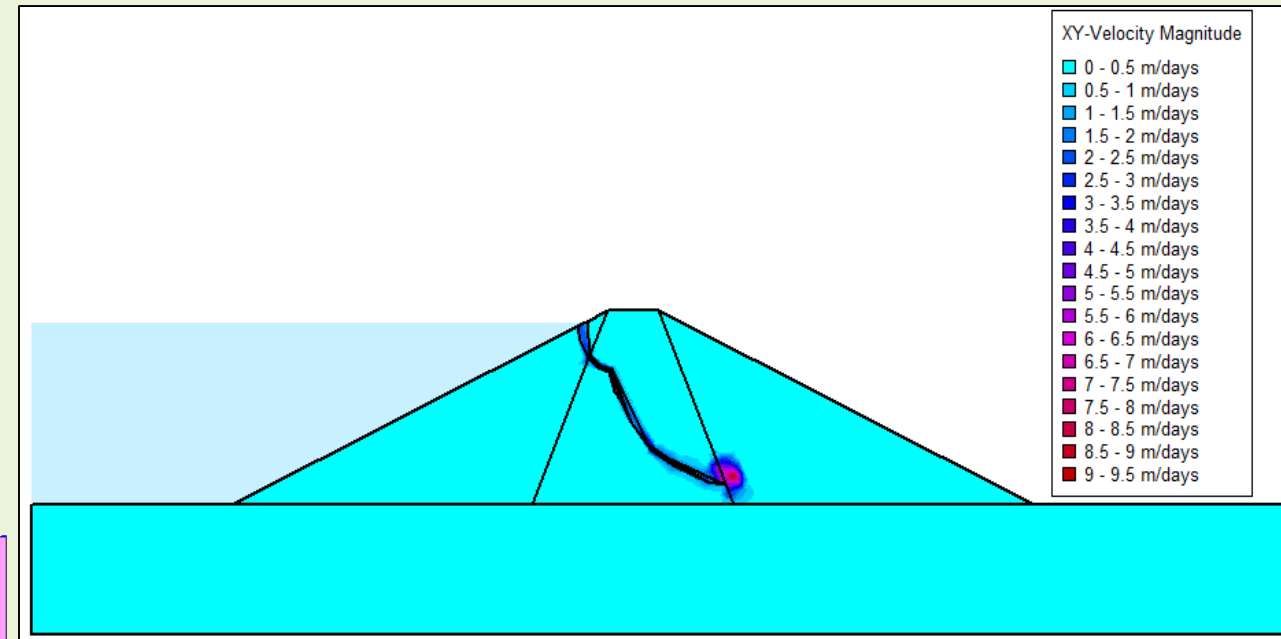
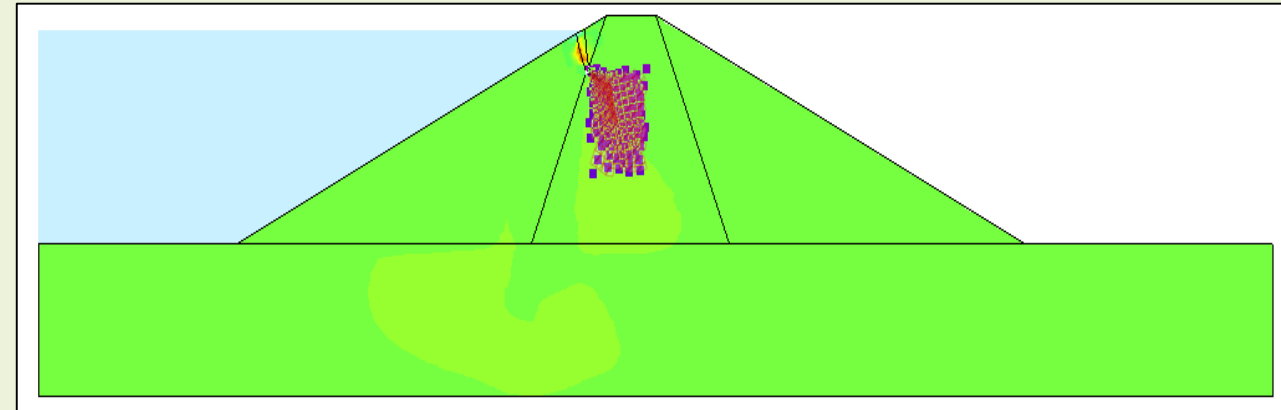
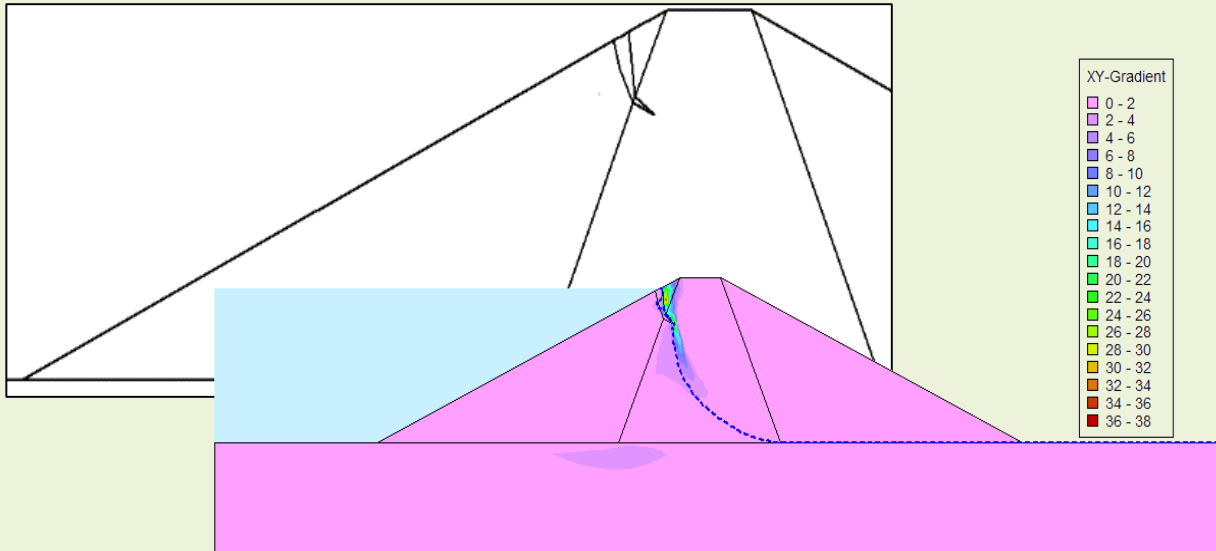
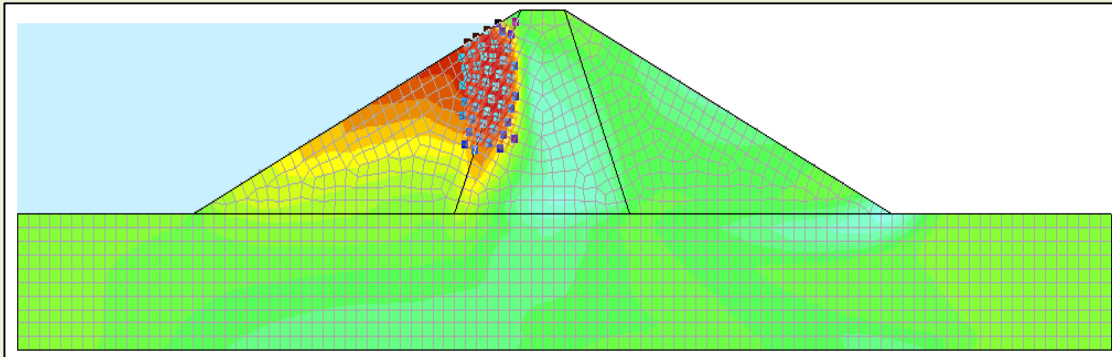
- Location of initiation of cracking
- Path of propagation of cracking



Hydraulic gradient contour on 38th day

Core-Cracking of Zoned Earthen Dams

- Location of initiation of cracking
- Path of propagation of cracking



Unreinforced Unpaved Roads

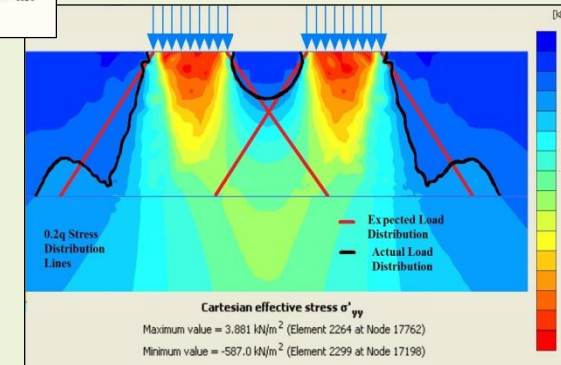
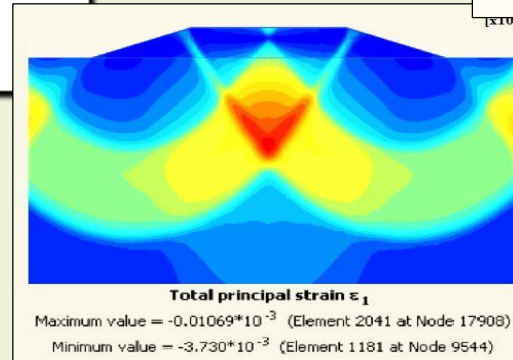
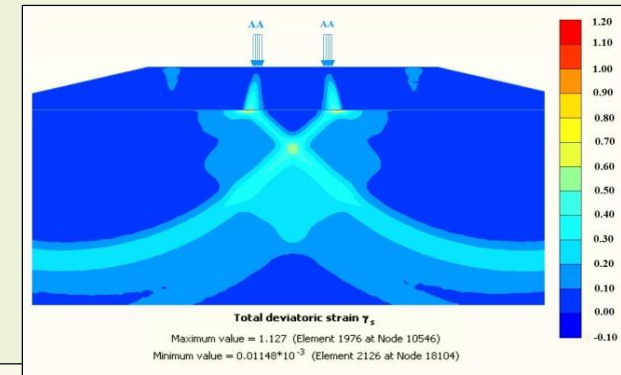
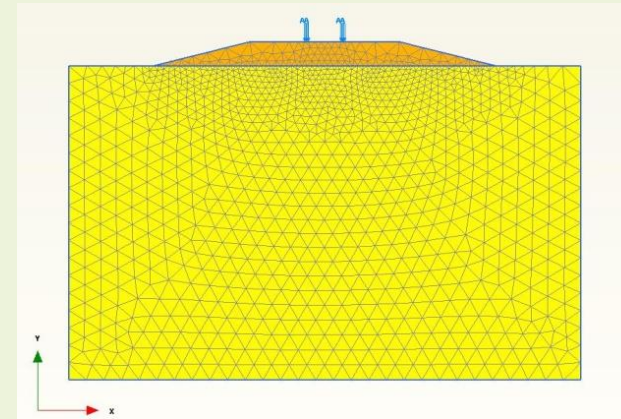
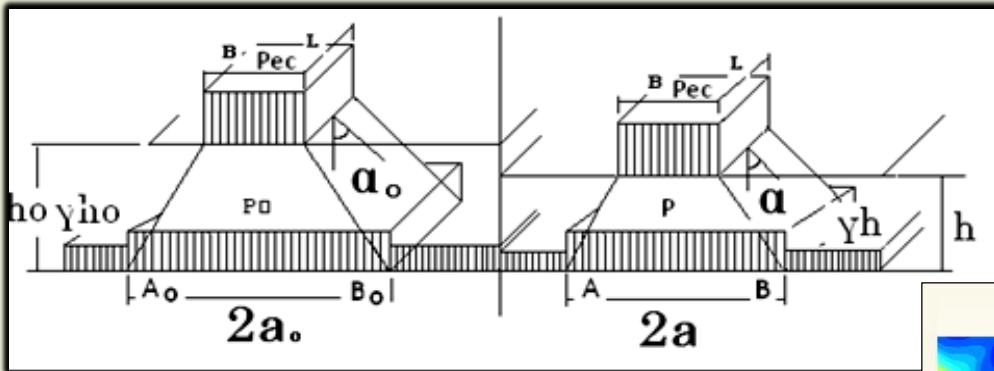
- Unreinforced unpaved roads
 - ❖ Unpaved roads resting on c-φ subgrade
 - ❖ Improvisation over Giroud and Noiray's technique
 - ❖ Finite element analysis using PLAXIS
 - ❖ Finite element based design of unpaved road
 - ❖ Percentage improvement using geosynthetics



Lokesh



Shivraj

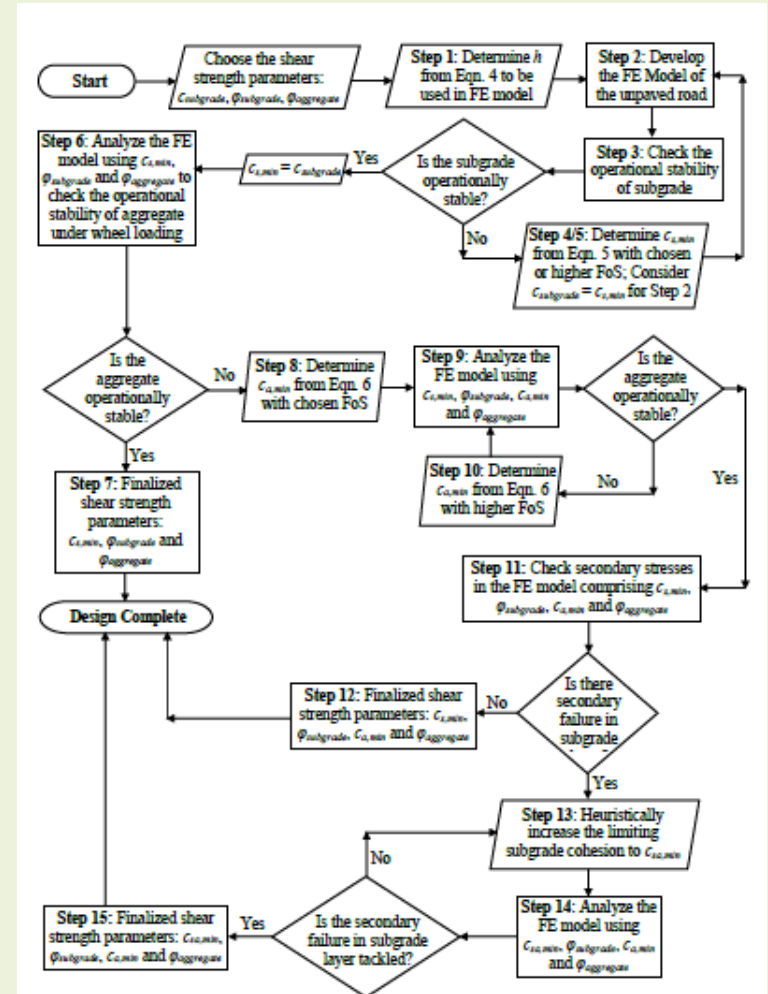
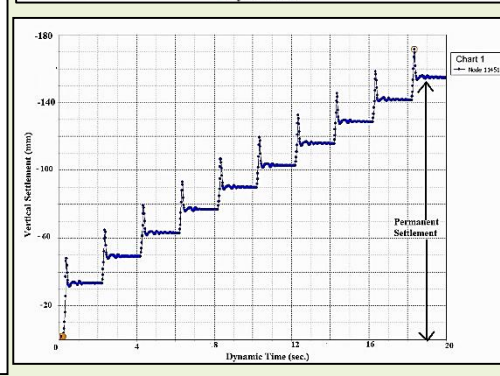
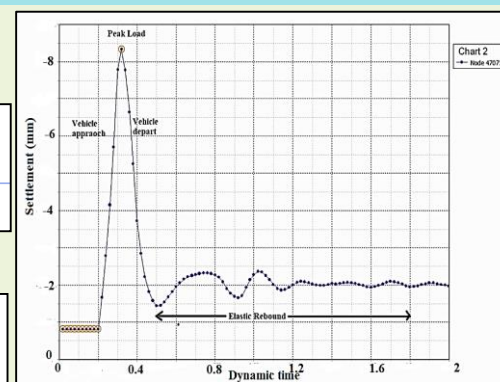
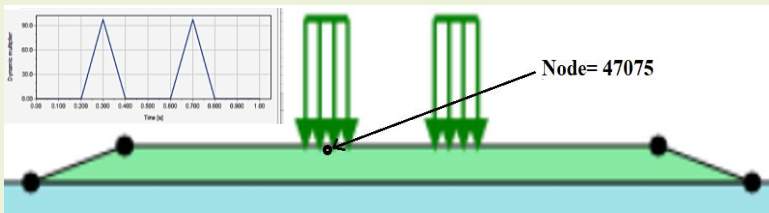
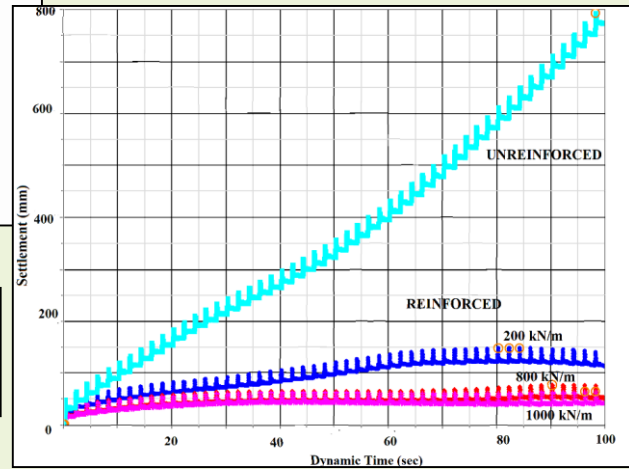
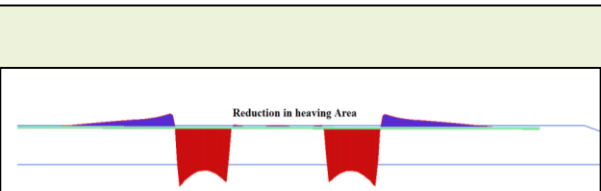
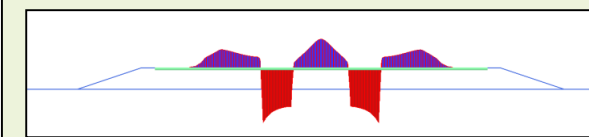
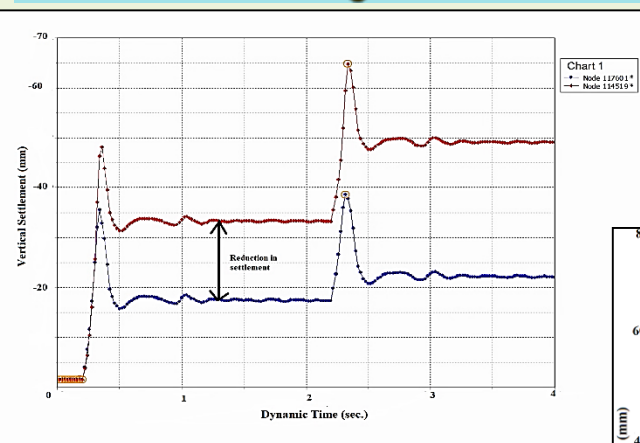
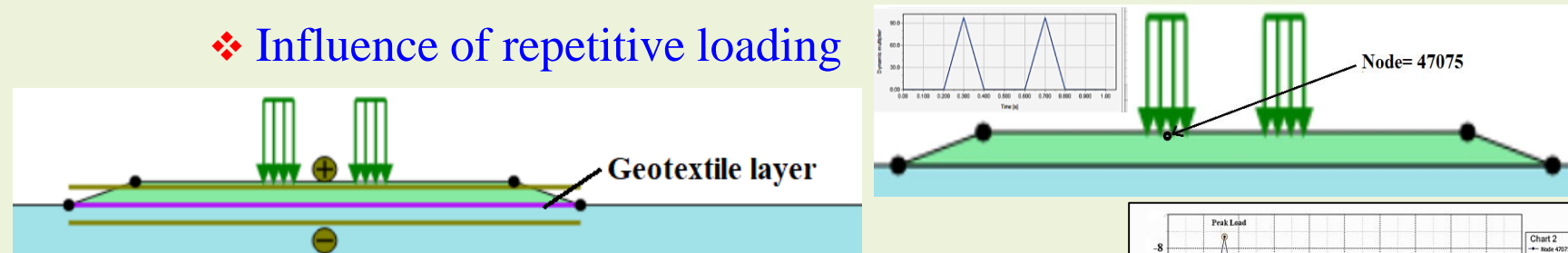




Nayan

Geosynthetic Reinforced Unpaved Roads

- Design of geosynthetic reinforced unpaved roads
 - ❖ Development of Finite Element based design algorithm
 - ❖ Influence of repetitive loading



Soft Ground Improvement using Preloading with PVDs

- Accelerated consolidation through radial drainage and excess PWP dissipation

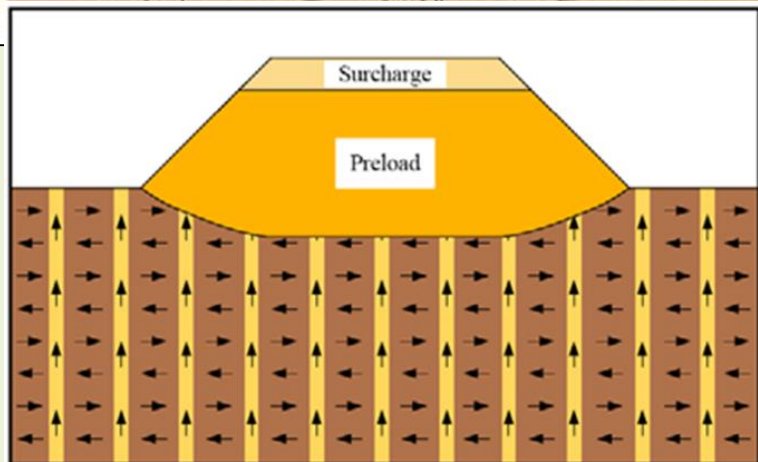
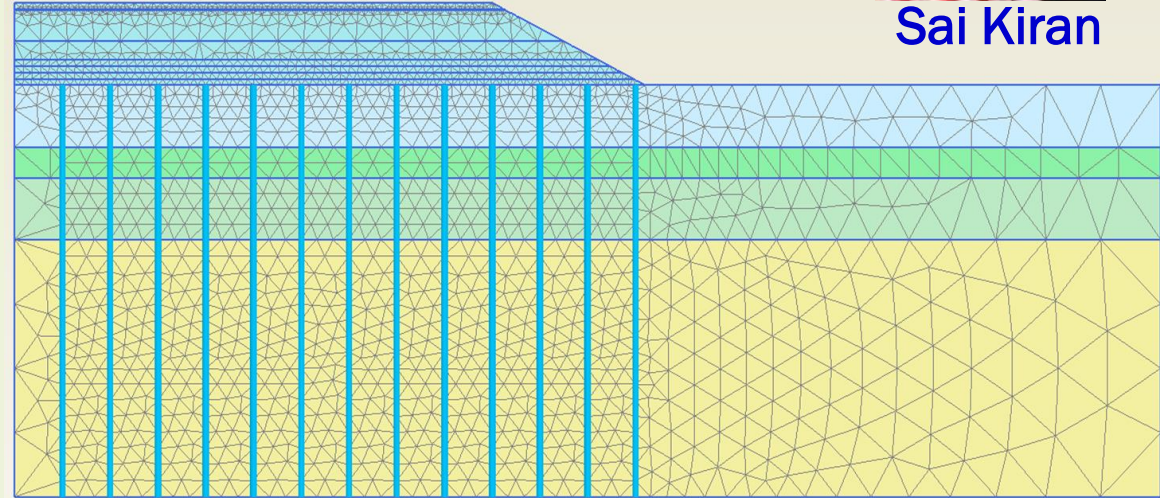
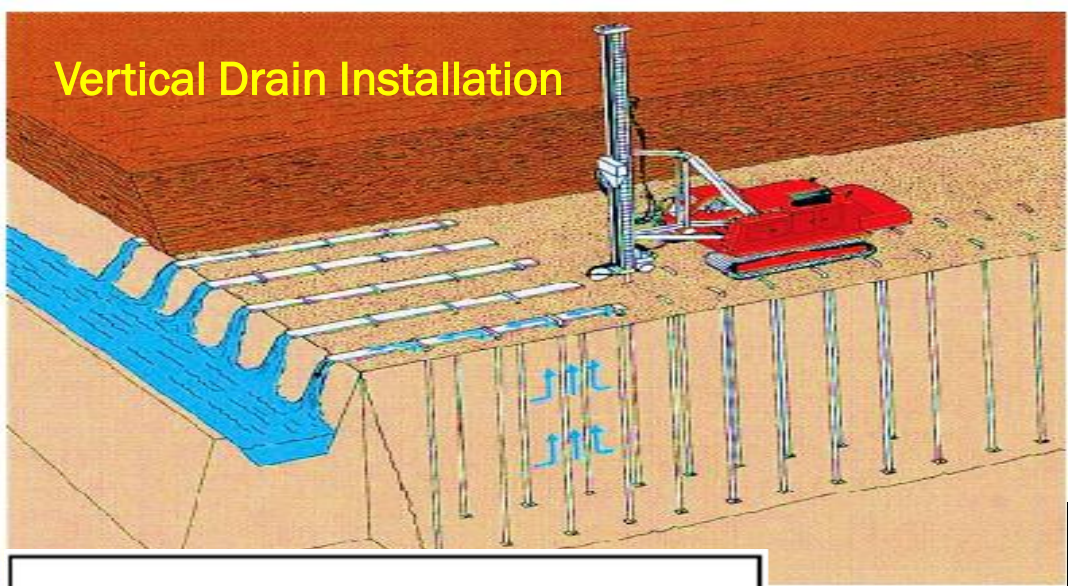


Sai Kiran

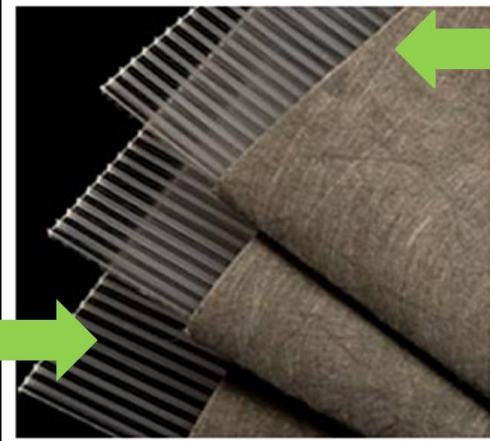


Rajesh

Vertical Drain Installation



Central core for to allow water to flow

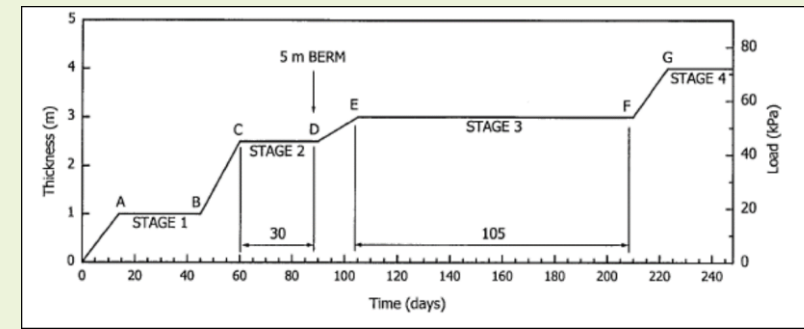


Plastic Core

Filter Sleeve

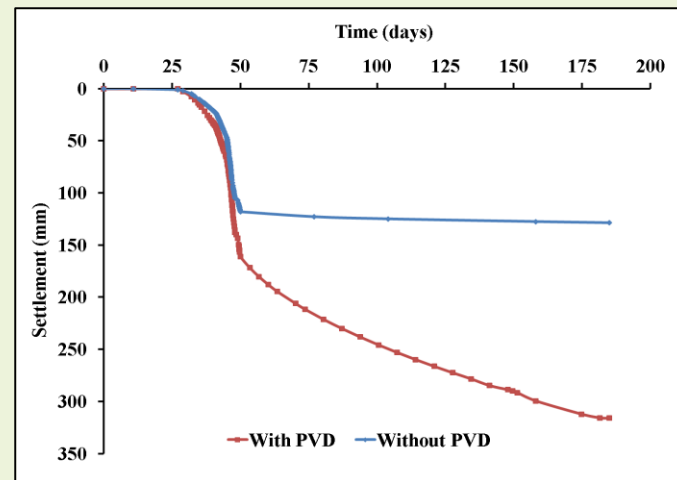
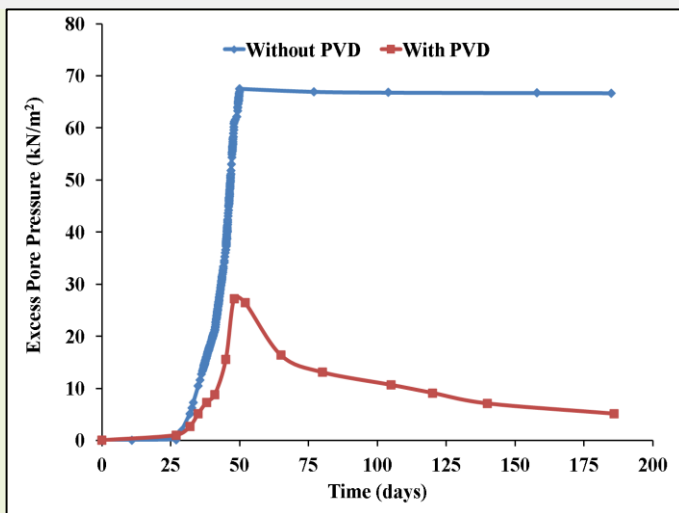
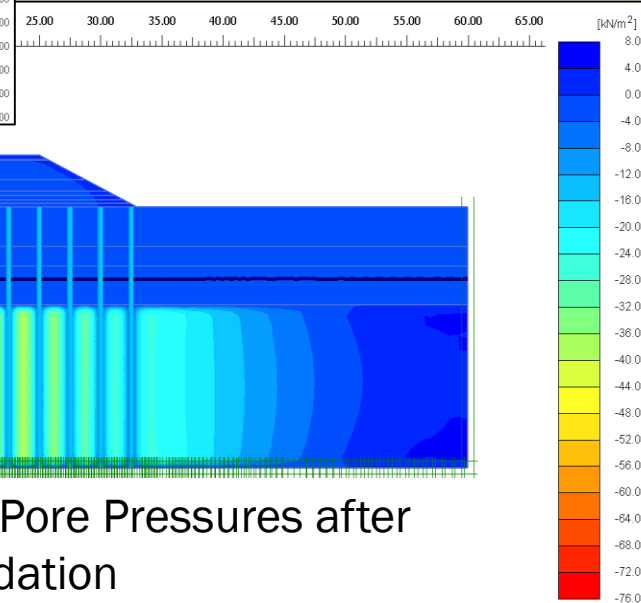
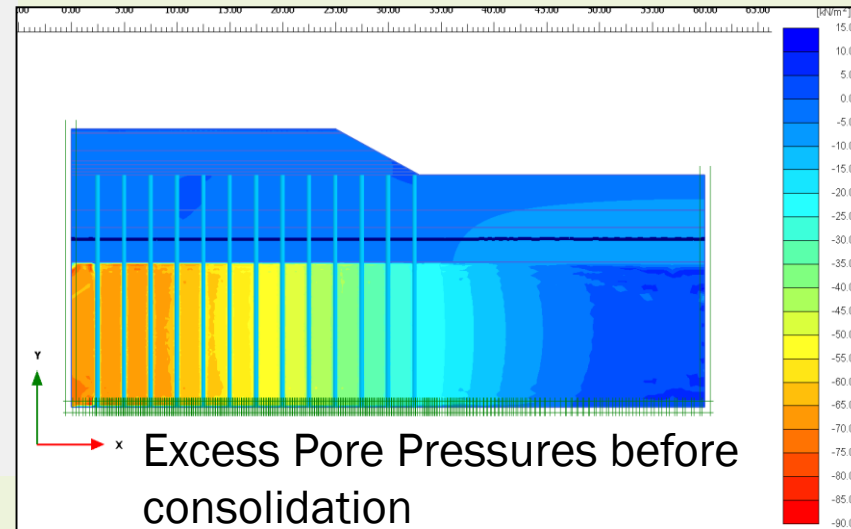
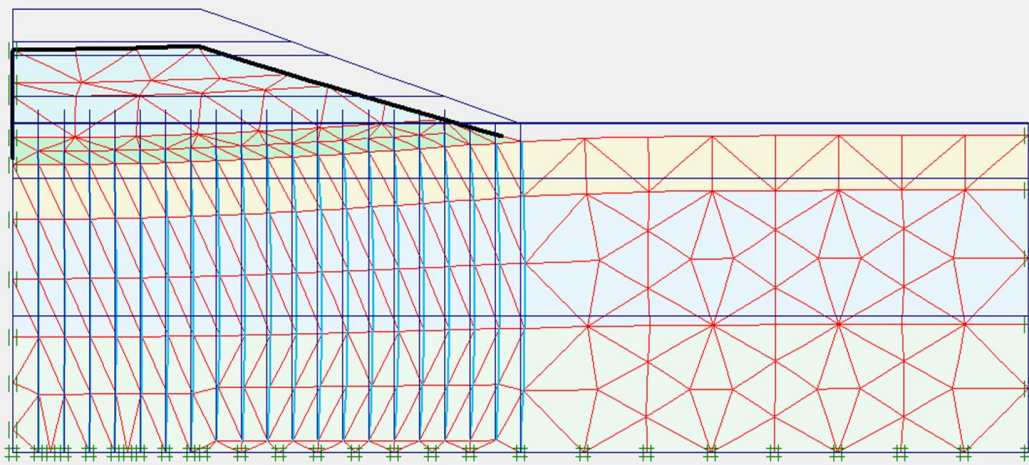
Filter jacket to prevent entry of soil particles

Typical PVD (Global Synthetics 2010)



Soft Ground Improvement using Preloading with PVDs

- Typical observations of ground improvement through PVD and Preloading



Heritage Railway Station, Udaipur, Agartala

- Application of preloading and PVD for developing of railway yard in a ditch marshland



Heritage Railway Station, Udaipur, Agartala

- Application of preloading and PVD for developing of railway yard in a ditch marshland



Other Ongoing Researches

- Research is a continuous effort to know the unknown
 - ❖ Seismic Response of Bridge Piers – Emte
 - ❖ Response of Single Pile Embedded in Inclined Stratigraphy – Debasmita
 - ❖ Soil Liquefaction and its Mitigation using Prefabricated Vertical Drains – Samrat
 - ❖ Reservoir Induced Seismicity – Anulekha
 - ❖ Hillslope stabilization using Geocells – Sureka
 - ❖ Risk Assessment of Earthen Dams and Embankments – Naveen
 - ❖ Engineering Behaviour of Varved Clays and Glaciolacustrine slopes – Deepali
 - ❖ Regional Scale Rainfall Induced Landslide Hazard Zonation – Manohara, Pooja
 - ❖ Seismic Microzonation – Aditya
 - ❖ Seismic Response Analysis of GRS and MSE Walls – Mihretab
 - ❖ Theoretical and Analytical Modeling of Complex Geotechnical Problems – Naina
 - ❖ Geosynthetics for Roadway Subgrades under Freezing-Thawing cycles - Chukhu



Thank You for Patient Hearing



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https://www.researchgate.net/profile/Arindam_Dey11